



Traffic, Transportation & Civil Engineering

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# **Traffic Impact Study**

Prepared For Polar Views, LLC

Residential Development

Located at
10 Grosvenor Street
Worcester, MA



September 2024

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# 1

### INTRODUCTION

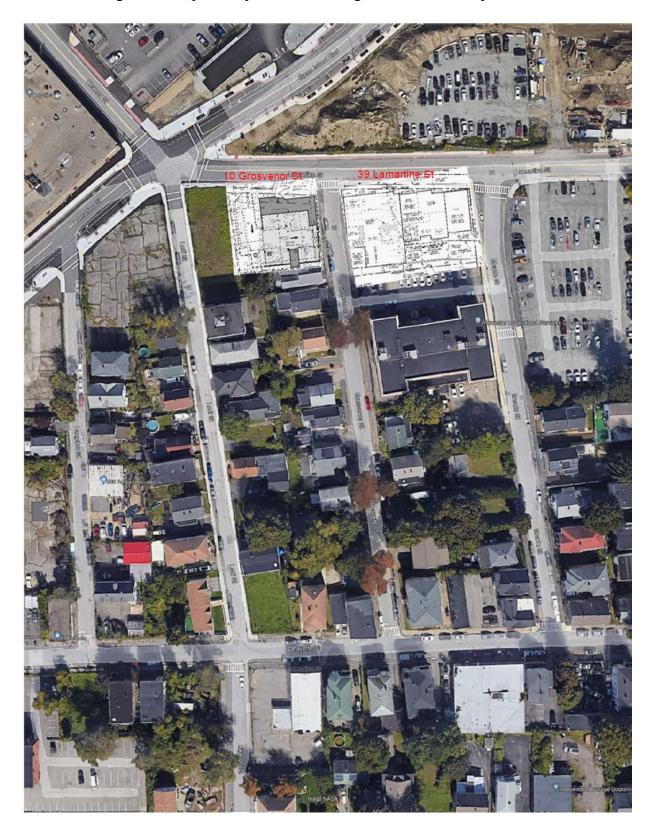
Polar Views, LLC, hereafter referred to as the applicant, is proposing the development of a parcel of land totaling 9,283 square feet (sf) to construct a four-story building housing a total of 12 apartment units. The proposed development is located south of Lamartine Street and west of Grosvenor Street in Worcester, Massachusetts. This development is next to another proposed development at 39 Lamartine Street which is located directly across Grosvenor Street. The applicant is proposing to evaluate the impact of this development on area roadway traffic and consider any improvements that may be necessary to make this development feasible and acceptable. This traffic study, which shares much of the data and analyses from the traffic study for the development at 39 Lamartine Street, is prepared to make this evaluation.

## **Project Description**

The applicant proposes to redevelop a 9,283-sf lot and construct a total of 12 apartment units. This parcel of land currently has a single-family residential structure on it. All proposed units will be accessed via an entrance-only driveway from Lamartine Street and an exit-only driveway on Grosvenor Street. These driveways will provide access to all proposed 10 off-street parking spaces. The Lamartine Street driveway is 12 feet wide while the Grosvenor Street driveway is 18 feet wide, both of which are equipped to provide one-way traffic circulation. Of the 10 proposed parking spaces, one will be designed and designated as handicap parking and will be located near the back entrance. Two of the parking spaces will be equipped with Electric Vehicle (EV) charging devices.

The proposed site is in the Business, General (BG-3.0) and Residence, General (RG-5) zoning districts and it currently has a single-family structure on it and its approximate location is directly across Grosvenor Street from the 39 Lamartine Street development project, which is shown in the aerial photograph in Figure 1.

Figure 1 - Proposed Apartment Building and Retail Development Site



# 2

## **EXISTING CONDITIONS**

Evaluation of the transportation impacts associated with the proposed residential development project, which is similar to but smaller than the development project at 39 Lamartine Street, requires a thorough understanding of the existing transportation system in the immediate vicinity of the site. Evaluating the existing study area streets operating conditions requires an examination of existing roadway traffic volumes, geometric features, and local community traffic-related issues. Each of these elements were evaluated and are described below.

## Study Area Roadway Network

In consultation with the city of Worcester DTM, the study area for this traffic impact report has been defined to include the traffic data and the evaluation of the same intersections as those for the project at 39 Lamartine Street and are listed below.

- Intersection of Lafayette Street and Grosvenor Street
- Intersection of Lafayette Street and Meade Street
- Intersection of Meade Street and Lamartine Street
- Intersection of Grosvenor Street and Lamartine Street
- Intersection of Lamartine Street, Lodi Street, Green Island Boulevard and Hermon Street

Additionally, per Worcester DTM, a total of eight Automatic Traffic Recorders were placed at the following locations.

- Across Hermon Street
- Across Lodi Street
- Across Lamartine Street east of Lodi Street
- Across Lamartine Street west of Lodi Street
- Across Green Island Boulevard
- Across Grosvenor Street
- Across Meade Street
- Across Lafayette Street

Lamartine Street is a two-way street with one travel lane in each direction. It intersects Quinsigamond Avenue at its terminus in a southwesterly direction and it intersects Millbury Street at its easterly end. It is flat and straight in the vicinity of the proposed site. The segment of Lamartine Street located between Quinsigamond Avenue and Green Island Boulevard that was recently reconstructed is 40 feet in width and it features bike lanes in both directions and on-street parking on the northeasterly side of the street. It is striped with double yellow center lines and its southwesterly approach at Quinsigamond Avenue is controlled by a fully actuated traffic signal system and its northeasterly approach at Green Island Boulevard is controlled by a stop sign. The section of Lamartine Street between Green Island Boulevard and Millbury

Street is 28 feet wide and was also recently reconstructed with new sidewalks and Americans with Disability Act (ADA) compliant handicap ramps. The north side of this section of Lamartine Street is posted with temporary no parking anytime signs, possibly for construction reasons. The approaches of this segment are also controlled by stop signs at Green Island Boulevard, Washington Street, Harding Street, and Millbury Street. This section is also striped with double yellow center lines. Finally, there are crosswalk markings across Lamartine Street at all its above-mentioned intersections. Land use along Lamartine Street is a combination of commercial and industrial uses.

**Meade Street** is a two-way street and traverses in the northerly and southerly directions. It has one lane of travel in each direction and its pavement width is 28 feet. It provides sidewalks on both sides of the street. It is 590 feet long and it intersects Lamartine Street to the north forming a "T" intersection and Lafayette Street to the south forming another "T" intersection. The land use on the southern half of the street is residential, consisting mostly of double and triple-deckers. The northerly half consists of the Worcester Department of Inspectional Services building on the west side and a parking lot serving that department as well as parking for Polar Park events on the east side. Presently, there are no on-street parking restrictions on Meade Street to accommodate the residential uses, except a dedicated handicap parking space in front of number 12 Meade Street, a 100-foot No Parking Anytime (NPA) on the west side of Meade Street from a point just north of the Worcester Inspectional Services Department building entrance in the northerly direction, and a distance of 250 feet NPA on the east side of the street from Lamartine Street southerly.

Grosvenor Street is also a two-way street with a pavement width of 28 feet. It also has sidewalks on both sides of the street. It runs parallel to Meade Street and its length is 590 feet. It intersects Lamartine Street to the north forming a "T" intersection and Lafayette Street to the south forming another "T" intersection. The land use consists of entirely residential double and triple-deckers, except the portion that abuts the Department of Inspectional Services. Presently, there are no onstreet parking restrictions on Grosvenor Street to accommodate the residential uses, except a dedicated handicap parking space in front of number 31 Grosvenor Street and one in front of number 46 Grosvenor Street.

**Lodi Street** is a one-way street traversing in the southerly direction. It has a pavement width of 20 feet, and it is approximately 600 feet long and parking is allowed on the west side of the street. It provides sidewalks on both sides of the street with proper crosswalks and handicap ramps at its intersections with Lamartine Street to the north and with handicap ramps only at Lafayette Street to the south. Finally, its southbound approach at Lafayette Street is controlled by a stop sign.

**Intersection of Meade Street and Lamartine Street** is a three-legged intersection, and its northbound approach is controlled by a stop sign, although it appears that the stop sign is missing. Its approaches are 14 feet wide except for the northbound approach that widens to 23 feet at the stop bar. There is a crosswalk across the northbound approach of Meade Street that is supplemented with a stop bar. This crosswalk is equipped with HP ramps that include tactile surfaces.

**Intersection of Meade Street and Lafayette Street** is also a three-legged intersection, and its southbound approach is controlled by a stop sign. Its approaches are 14 feet in width and there is a

crosswalk spanning across Meade Street that also has a stop bar. Again, this crosswalk is equipped with HP ramps that include tactile surfaces.

Intersection of Lamartine Street, Green Island Boulevard, Hermon Street and Lodi Street is a five-legged intersection located approximately 200 feet west of the Lamartine and Grosvenor Streets intersection. Of the five streets diverging at this intersection, Lodi Street is a one-way street traversing in the southerly direction, thus, for all intents and purposes, it operates as a four-legged intersection. Its northeasterly and southwesterly approaches are 16 feet in width and feature a bike lane. The Lamartine Street approach is 14 feet wide, while the Hermon Street approach is 12 feet wide in addition to a 2-foot shoulder. All legs of this intersection are marked with crosswalks and associated HP ramps that include tactile surfaces. All legs of this intersection except the Lodi Street leg, which is a one-way street in the southerly direction, are controlled with stop signs and are marked with stop bars.

**Intersection of Grosvenor and Lamartine Streets** is a three-legged "T" intersection and its northbound approach, which is 14 feet wide, is controlled with a stop sign, but the stop sign is missing. As stated herein above, Lamartine Street is a two-way street and on-street parking is allowed on the north side of the street. There is a crosswalk across Grosvenor Street supplemented with a stop bar.

**Intersection of Grosvenor and Lafayette Streets** is also a three legged "T" intersection and its southbound approach, which is 14 feet wide, is controlled by a stop sign, again, it appears that the stop sign is missing. Lafayette Street is a one-way street in the easterly direction and its easterly Street approach is 15 feet wide with parking allowed on both sides of the street. There is a crosswalk across Grosvenor Street.

## **Traffic Volumes**

New peak hour turning movement counts (TMCs) were collected for the 39 Lamartine Street projects on Wednesday, July 17<sup>th</sup> and Thursday, July 18th, during two-hour periods between the hours of 7-9 AM and 4-6 PM commuter peak periods in order to identify the critical peak hour. The following Figure 2 shows the baseline peak hour traffic counts conducted on the above dates for all five intersections. Also, Table 1 below shows the daily traffic, AM and PM peak hour traffic, average (Mean Speed), and 85<sup>th</sup> percentile speed at eight locations within the study area.

Table 1 Automatic Traffic Recorders Results

Street	Lamartine St E	Lamartine St S	Meade St	Green Island St	Grosvenor St	Hermon St	Lafayette St	Lodi St
ADT	3,902	3,271	129	2,472	235	1,111	889	289
AM Peak Vol	268	146	21	197	18	134	61	28
PM Peak Vol	310	256	16	218	32	185	65	33
Average Speed mph	20.6	29.5	13.5	21.8	15.6	21.5	19.2	15.2
85th% Speed mph	24	34	18	24	20	25	20	16

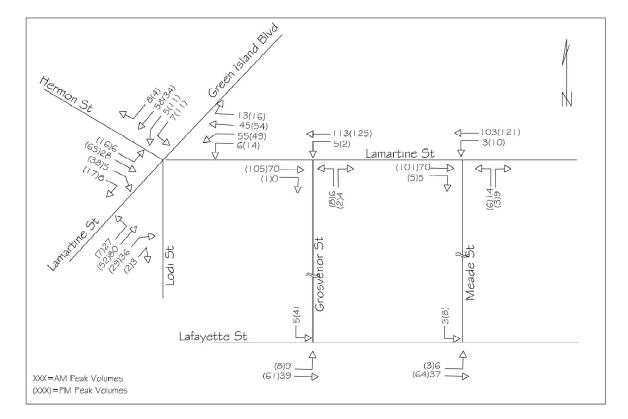


Figure 2 – Baseline Turning Movement Counts

The *massDOT* has provided updated guidance that no longer requires pandemic-related adjustment of traffic counts performed after March 2022 except in locations where the predominant land use consists of offices or other similar uses. Therefore, given the predominant land use in the study area is not of office use, traffic volumes traveling through these intersections were not adjusted.

Traffic Volume and Classification, the streets within the study area are included in the group U4-U7 for Growth Factor and Seasonal Factor. To evaluate the potential for seasonal fluctuation of traffic volumes on roadways near the proposed site, weekday seasonal factors were obtained from the *massDOT* Statewide Traffic Data Collection. The review of the *massDOT* seasonal adjustment factors shows that roadways having similar characteristics to roadways within the study area (U4-U7) have a factor of 0.92 for traffic counts collected in the month of July. However, because this factor results in a reduction in the volumes, no adjustments were made in order to assess the worst-case scenario. Therefore, the TMCs were not adjusted by this factor. A copy of the *massDOT* seasonal adjustment factors is included in the Technical Appendix section of this report. The baseline (year 2024) traffic volumes for the AM and PM peak periods are shown in Figure 2 above.

Typically, the PM peak period has the higher volumes, and is considered the critical peak. As is

the case here, higher traffic volumes through the study area intersections also occur during the PM peak period.

## Safety Concerns

**Sight Distances:** To evaluate the safety of traffic to and from the site via its driveways, sight distances were measured in the field and analyzed.

Sight distance is defined in the *massDOT* Project Development and Design Guide as the length of roadway ahead that is visible to road users. In most cases, specific sight distance measures apply to motor vehicles and bicyclists. There are two aspects of sight distance that apply to this case. They are:

- Stopping sight distance
- Intersection sight distance

The sight distances are related to the design speed (posted speed) of the roadway and are based on the standards of the American Association of State Highway and Transportation Officials (AASHTO) *A Policy on Geometric Design of Highways and Streets*, also referred to as the *Green Book*.

Because the proposed site exit-only driveway is 65 feet from the intersection of Lamartine and Grosvenor Street, vehicles traversing by this driveway are at a near stop speed, therefore, the intersection site distance does not apply to this site.

**Stopping Sight Distance** is further described in the *massDOT* Project Development and Design Guide as the distance necessary for a vehicle traveling at the design speed (posted speed) before reaching a stationary object in its path. The sight distance at every point along a roadway should be at least the stopping sight distance.

The sight distance for vehicles leaving the site via the Grosvenor Street exit-only driveway to the right (south) was measured in the field. The measured distance is from a point five feet back of a stop bar (approximately 15 feet from the street line) and 3.5 feet above grade to represent drivers' eye height to an object 3.5 feet above roadway grade. The field review of Grosvenor Street showed that the available sight line to the left (north) for vehicles exiting the site driveway is 65 feet all the way through its intersection with Lamartine Street. The field review of Grosvenor Street showed that the available sight line to the right (south) for vehicles exiting the site driveway is 290 feet. As stated earlier, there are no posted speed limit signs on Grosvenor Street. Therefore, the statutory prima facie speed limit of 30 miles per hour applies to this street.

Based on Basic Design Controls for roadway design, the Stopping Sight Distance is calculated using the formula d=(V\*V)/(30\*f), plus the time required for perception and reaction by a driver (2.5 seconds). V is approach speed in mph, and f=0.28-0.35. The stopping sight distances are calculated and are provided in Exhibit 3-8 of the 2006 *massDOT* Project Development and Design Guide. A

copy of this exhibit is presented in the Technical Appendix section of this report. Due to the less than 1% grade of either street, the required stopping sight distance for 30 mph is 200 feet for points south of the proposed site for both streets. The sight distance was also examined vertically. The following aerial photo in Figure 3 illustrates the profile of Grosvenor Street in the vicinity of the proposed exit driveway. The available sight distances for the proposed exit-only driveway is also shown visually in the photographs in Figure 4.

It should be noted that the proposed driveway is only 65 feet from the Lamartine Street intersection. Therefore, vehicles either arriving at (northbound) or approaching from (southbound) Lamartine Street must make a right or left turn maneuver at a very low speed or from a stop. As a result, vehicles moving in either direction from both streets are not expected to reach speeds higher than 10 mph. Due to the less than 1% grade of either street, the required stopping sight distance for 10 mph is less than 100 feet. As demonstrated herein above, the available sight distances are greater than the standard values for vehicles traveling at 10 mph passing the site driveway. Therefore, proper sight distances will be provided.

In conclusion, driveways in an urban setting like the proposed site are not subjected to follow such standards particularly since these streets are short in length and on-street parking is allowed, thus not affording motorists the opportunity to reach the speed limit. As demonstrated in Table 1, the median speed on Grosvenor Street was measured at 15.6 mph and the 85<sup>th</sup> percentile speed was 20 mph.

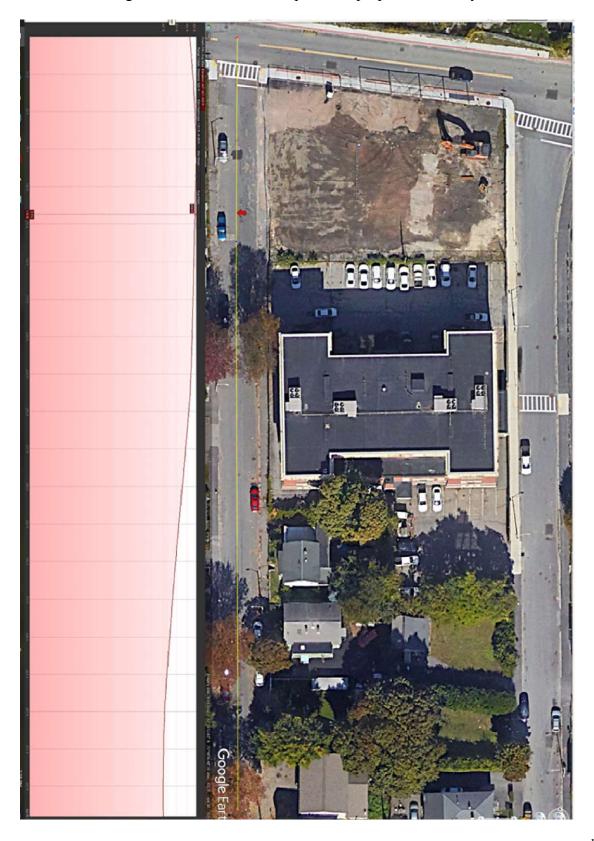


Figure 3 – Grosvenor Street profile at proposed driveway

Figure 4
Views from proposed Grosvenor Street driveway looking to the right (south) and left (north)



Accidents: The results of the accident analysis conducted for the 39 Lamartine Street project were used for the proposed site. The latest accident data compiled by the *massDOT* were obtained and reviewed for a five-year period of January 1, 2019-December 31, 2023. This review summarizes the total number of accidents that occurred at each of the five intersections within the study area during this period, and the summary is listed in Table 2, below. It is noted that a total of three accidents occurred during this period at the intersection of Lamartine Street, Hermon Street, Lodi Street and Green Island Boulevard, while a total of 2 accidents were reported during the same period for the intersection of Lafayette Street and Meade Street. None of the accidents at these two intersections involved injuries. Also, none of the accidents took place during peak traffic periods. The breakdown of all accidents at these two intersections is also presented below in Table 2. Finally, the remaining three intersections had no accidents reported during this five-year period.

Using the baseline turning movement counts compiled during traffic surveys of these five intersections, accident rates were calculated in accidents per million vehicles entering each intersection. Utilizing the *massDOT* prescribed methodology, the accident rates for these intersections were calculated and compared with the *massDOT*'s latest available rate of 0.61 for unsignalized intersections on roadways in District 3 of the *massDOT*, in which the City of Worcester is located. A summary of the accident rates is also included in the following Table 2. A copy of the accident rate calculation sheets is included in the Technical Appendix section of this report. Also included in the Technical Appendix section of this report is a copy of the *massDOT* Average Crash Rates for signalized and unsignalized intersections throughout the Commonwealth of Massachusetts.

The above accident analysis indicates that the rate of accidents for the intersection of Lamartine Street, Hermon Street, Lodi Street and Green Island Boulevard is significantly lower than the *massDOT* rate of 0.61. However, the accident rate for the intersection of Lafayette Street and Meade Street is much higher than the *massDOT* average. Both accidents at this location were of angle type, suggesting a potential lack of sufficient sightline for vehicles making a left-turn maneuver from Meade Street.

Table 2 - Vehicle Crash Summary for 5 Years (Jan 1, 2019-Dec 31, 2023)

		•	`	•	
	amartine Meade St	Lamartine Grosvenor	Lamartine Green Island	Lfayette Grosvenor	Lafayett Meade
Intersection U	Insignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalize
Calculated Crash I	Rate 0	0 massDOT Av Rate	0.35 0.61	0	1.32
Year					
2019	0	0	1	0	0
2020	0	0	1	0	1
2021	0	0	0	0	0
2022	0	0	0	0	1
2023	0	0	1	0	0
Total	0	0	3	0	2
Collision Type					
Angle	0	0	2	0	2
Head-On	0	0	0	0	0
Rear-end	0	0	0	0	0
Sideswipe	0	0	0	0	0
Single Vehicle	0	0	1	0	0
Unknown	0	0	0	0	0
Total	0	0	3	0	2
Severity					
Fatal Injury	0	0	0	0	0
Non-Fatal Injury		0	0	0	0
Property Damage		0	3	0	2
Total	0	0	3	0	2
Time of Day					
7:00 AM to 9:00		0	0	0	0
4:00 PM to 6:00		0	0	0	0
Other Times	0	0	3	0	2
Total	0	0	3	0	2
Pavement Condit					
Dry	0	0	1	0	2
Wet	0	0	1	0	0
Snow/Ice	0	0	1	0	0
Total	0	e: massDOT Crash Portal	3	0	2

## **Existing Conditions Summary**

The proposed site consists of a vacant single-family structure on a parcel of land located at the southwest corner of the intersection of Lamartine Street and Grosvenor Street. The applicant proposes to build a four-story building to house a total of 12 apartment units and provide a total of 10 off-street parking spaces. Of the 10 parking spaces, one will be designed and designated as a van accessible handicap parking space. Also, two of the parking spaces will be equipped with charging devices for Electric Vehicles (EV). Finally, a 50-foot-long loading space will be provided on the west side of the property near the entrance to the site.

Lamartine Street can be characterized as a two-lane roadway with one travel lane in each direction along its entire length. The segment of Lamartine Street between Quinsigamond Avenue and Green Island Boulevard also provides bike lanes as well as on-street parking on one side of the street. The segment between Green Island Boulevard and Millbury Street, that the proposed site also fronts on, is 28 feet wide and is posted with No Parking Anytime (NPA) signs on the south side and temporary NPA signs on the north side of the street. Sidewalks are provided on both sides of the entire length of Lamartine Street. It is a local street that serves many types of land uses, including industrial, office and residential properties.

Similar to Lamartine Street, Grosvenor Street is also a two-way street with a pavement width of 28 feet. It also had sidewalks on both sides of the street. It runs parallel to Meade and Lodi Streets and its length is 590 feet. The land use consists of entirely residential properties, except the portion that borders the Department of Inspectional Services. Presently, there are no on-street parking restrictions on Grosvenor Street to accommodate the residential uses, except a dedicated handicap parking space in front of number 31 Grosvenor Street, and one in front of number 46 Grosvenor Street.

The proposed development project is located in Residence General (RG-5) and Business, General (BG-3.0), and is currently vacant.

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## **FUTURE CONDITIONS**

Traffic volumes in the study area are projected to post-development levels. Projected traffic volumes include the existing traffic data obtained from the turning movement counts adjusted to represent the baseline, projected into the future year 2029 peak hours to reflect increases due to future area projects or background growth, and added to the new traffic expected to be generated by the proposed residential development site.

## Site-Generated Traffic

The number of trips that will be generated by the proposed 12-unit apartment building was projected by using the latest *Trip Generation Manual* published by the Institute of Transportation Engineers (ITE).

Based on the ITE *Trip Generation Manual*, the rates at which the proposed land uses generate traffic vary depending upon the time of day. These rates were used to calculate the number of trips expected to be generated by the proposed 12-unit apartment development during an average weekday, as well as morning and afternoon peak traffic periods. The ITE trip generation charts are presented in the Technical Appendix section of this report. The resulting trips and their directional distribution for this site are shown in the following Table 3.

TABLE 3 Trip Generation and Distribution for 10 Grosvenor Street

12 Apartments ITE Land Use Code 221

Rate-Trips/Unit Trips

12 Aparan	CITCS TIL Lai	10 030 000	, 221					
Daily	%In	%Out	AM Pk	%In	%Out	PM Pk	%In	%Out
2.93	50%	50%	0.28	15%	85%	0.26	73%	27%
35	2.93 50% 50		3	1	2	3	2	1

As can be seen in the above Table 3, the total number of new trips expected to be generated by the proposed residential development are equal during both peak periods. Therefore, both peak traffic periods were evaluated for this development.

## **Trip Distribution and Assignment**

The projected new site-generated trips from the above Table 3 are assigned to each approach of the intersections within the study area. As shown in Table 3 above, during AM peak period, one vehicle will be arriving at the proposed site via the Lamartine Street entrance and two vehicles will be departing from the site in the northerly and southerly directions from the exit-only driveway along Grosvenor Street. Similarly, during PM peak period, two vehicles are expected to arrive via the Lamartine Street entrance and one vehicle will depart from the proposed site's Grosvenor Street exit-only driveway. Finally, a total of 17 vehicles will be arriving and 18 vehicles will be leaving

from the proposed site during a 24-hour period on an average weekday. The following Figure 5 shows the above-mentioned distribution of trips associated with the proposed site along the streets in the study area and the site driveways.

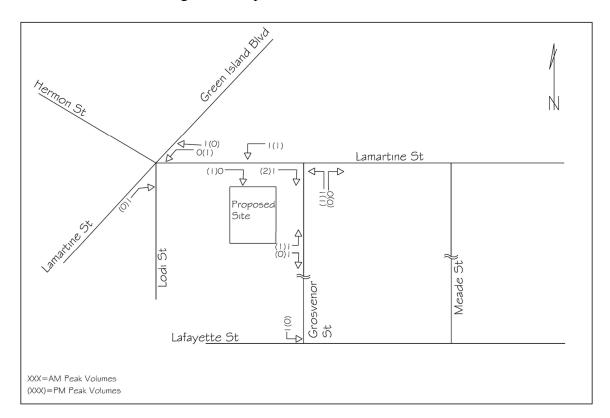


Figure 5 - Trip Generation and Distribution

## Site Access, Circulation and Parking

Site access and internal traffic circulation were evaluated as part of assessing the proposed residential development. Access to the proposed site off-street parking facility will be achieved through an entrance-only driveway from Lamartine Street and an exit-only driveway from Grosvenor Street. The proposed driveways will provide a one-way traffic circulation with full access to all parking spaces. The Lamartine Street entrance-only driveway will have a pavement width of 12 feet while the Grosvenor Street exit-only driveway will have a pavement width of 18 feet, both of which are designed to accommodate one-way traffic. One advantage of a one-way circulation is the reduction in the number of vehicular conflict points, thus improving safety. Finally, the Grosvenor Street exit-only driveway will be posted with an MUTCD standard stop sign and marked with a stop bar, as well as a Do Not Enter sign as shown on the site plan.

The magnitude of parking spaces that will be needed for the proposed residential development was

projected by using the latest (6<sup>th</sup>) edition of the *Parking Generation Manual*, also published by the ITE. A copy of the relative page is included in the Appendix section of this report.

Based on the ITE *Parking Generation Manual*, the rates at which mid-rise multifamily residential developments generate demand for parking vary depending upon the location of the project. The demand for off-street parking is highest for facilities located in suburban areas primarily due to the lack of public transportation and long distances from daily conveniences. Based on the ITE *Parking Generation manual*, the 85<sup>th</sup> percentile or peak period parking demand rate for mid-rise multifamily residential developments in dense multi-use urban areas not close to rail transit is 1.29 parking spaces per dwelling unit. However, the average parking demand is 0.93 parking spaces per dwelling unit. As stated earlier, a total of 10 parking spaces are proposed for this site. Thus, the proposed parking supply is calculated at 0.83 spaces per unit. The proposed number of parking spaces is 12% lower than the national average rate of 0.93 and 50% lower than the 85<sup>th</sup> percentile (peak) demand on a weekday. Given the Transportation Demand Management program highlighted below, it is anticipated the actual demand for off-street parking should be lower than that of the ITE *Parking Generation manual* during 85<sup>th</sup> percentile (peak) demand.

Therefore, based on the above assessment, it is concluded that sufficient parking spaces will be provided.

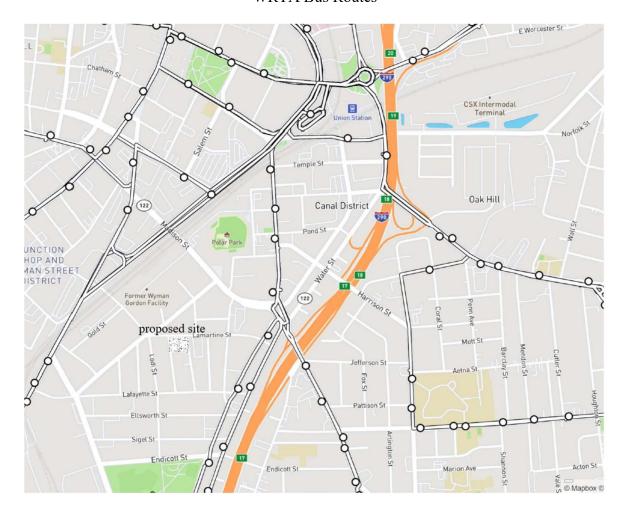
## **Transportation Demand Management**

The proposed development at 10 Grosvenor Street, which is located directly across Grosvenor Street from the 39 Lamartine Street development project, and which includes a total of 12 apartment units, will benefit from many of the city conveniences when it comes to different modes of transportation. The proposed site is located approximately a half mile from the Transportation Hub that provides access to all Worcester Regional Transit Authority (WRTA) bus routes, as well as Greyhound intercity busses, and it is also connected to Worcester Union Station at Washington Square that provides services via Amtrak and the MBTA to points east, including the City of Boston. Thus, this TDM program is largely geared toward the use of these public transportation services and other modes such as pedestrian walkability, bicycling, etc. Therefore, it is suggested the consideration of advancement of the following improvements as part of the Project, which are commensurate with the predicted impact of the Project on the transportation infrastructure and are focused on safety and encouraging the use of alternative modes of transportation to single-occupancy vehicles:

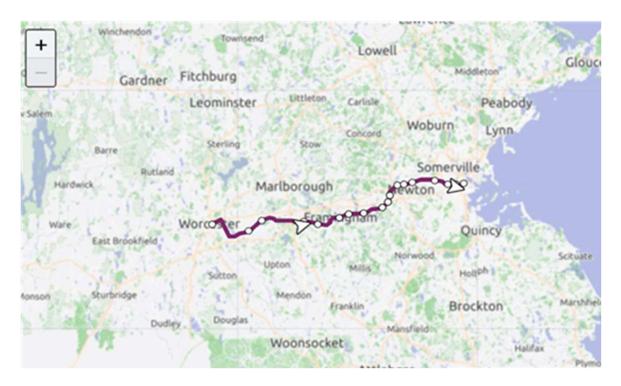
**Public Transportation** – is a large component of this TDM program. There are WRTA bus stops located within 4-5 minutes walking distance of the proposed site at 10 Grosvenor Street. Since this area of the Canal District section of the city is being developed with many new residential uses, and since the surrounding streets have sidewalks, this distance could be further reduced to less than a two-minute walk by providing additional bus stops on Green Island Boulevard and/or Lamartine Street. Therefore, WRTA should be consulted about establishing additional bus routes and bus stops in the general area of this and other future developments. Currently, there are bus stops that include WRTA Route 4 with 15-minute headways, two of which are located on Harding Street at Lafayette Street

and Lamartine Street for inbound direction, and one is located on Millbury Street at Lafayette Street for outbound direction. The residents of the proposed development could walk 10-13 minutes directly to the Transportation Hub or Union Station to transfer to other parts of the city or for commuting purposes particularly to points east, as far as the City of Boston by utilizing the MBTA train services. Therefore, the need for owning a motor vehicle and available off-street parking facilities is minimal. Also, to further discourage the use of personal vehicles, it is recommended that all information about the abovementioned public transportation services such as maps of bus routes, bus and MBTA schedules, and fares should be made available to the future residents of the proposed apartments. Additionally, it is recommended that WRTA be consulted to provide a dynamic monitor placed either inside or outside visible to the residents of both 39 Lamartine Street and the proposed site which will display live information relative to available WRTA routes through the area. Below is a bus route map that shows the existing bus stops in the vicinity of the proposed development as well as an MBTA map showing the stops along its route to Boston.

#### WRTA Bus Routes







Walkability – is considered a major part of Healthy Transportation mode and an advantage to the future residents of the proposed development at 10 Grosvenor Street, particularly since all streets to and from the proposed development site have sidewalks and proper handicap ramps at their intersections. The proposed site is bound by Lamartine Street to the north, Grosvenor Street to the east, and Lodi Street to the west. Lamartine Street leads to Harding and Millbury Streets in the easterly direction and to Green Island Boulevard and Hermon Street in the westerly direction. All of the above-mentioned streets provide proper sidewalks and handicap ramps making travel on foot to many points of interest practicable. Therefore, no additional pedestrian accommodation is warranted, further minimizing the need for owning personal motor vehicles and off-street parking facilities.

Bicycling – is also considered a Healthy Transportation mode for the future residents of the proposed site at 10 Grosvenor Street, particularly since several of the streets in proximity of the proposed site feature dedicated bike lanes such as Lamartine Street between Quinsigamond Avenue and Green Island Boulevard, Green Island Boulevard, Harding Street, and Millbury Street. Some other nearby streets that do not have dedicated bike lanes may also be candidates for being marked with Sharrow symbols to let motorists know they have to share the road with bicyclists. Therefore, to further reduce the demand for motor vehicle ownership and on/off-street parking, the proposed site should provide a secure indoor and outdoor bicycle parking facility to accommodate 4-6 bicycles.

Accessible Parking – should be an important component of this TDM program as some residents will ultimately own personal motor vehicles that will need off-street parking accommodation. Although this demand will be significantly lower than in other locations with lower density and less accessibility to public transportation, they will require off-street parking to a lower degree. On-street parking should be made available for use by some of the residents of the proposed site, even if it is for short-term parking to allow passenger pick-up/drop-off and loading and unloading groceries. Presently, on-street parking is allowed on the west side of Lodi Street, except for a dedicated handicap parking space in front of number 5 Lodi Street. Parking is not allowed on the east side of Lodi Street as it is a one-way Street in the southerly direction. Finally, to better manage the available on-site parking and to further discourage the need for personal car ownership, the applicant may have to assign each space only to the few residents who may own a vehicle or even charge a premium for off-street parking. It should be noted that in case of the need for on-street parking for the purpose of moving in/out of these apartments, a special consideration could be obtained from the Parking Control section of the City's DTM's.

**Ride-Sharing Mode** – is a valuable form of transportation that could further reduce the need for personal vehicle ownership, and thus, reduce traffic on area streets as well as decrease the need for on and off-street parking facilities. On-street parking is currently available on most of all three streets bordering the proposed site at 10 Grosvenor Street that could be used for ride sharing purposes. Therefore, ride-sharing vehicles can use the nearby on-street parking to pick up/drop off passengers, as they are only short-term parking. Ride-share vehicles can also use the internal off-street parking spaces if on-street parking should not be available. Consequently, no special parking space designation would be necessary.

**Work-at-Home** – accommodations should be included within the proposed project. Although the COVID-19 pandemic has ended, a sizeable portion of the workforce continues to work from home as both employers and employees have become accustomed to the practice of the work-at-home concept. Therefore, the work-at-home accommodation may take the form of providing a meeting space or potentially a business office in the common area with such amenities as available internet, a computer and a printer.

**Transportation Coordinator** – or a building superintendent would be an asset for the new residents of the proposed development. The coordinator or superintendent who may also have other responsibilities, would coordinate all components of the above-mentioned TDM program such as providing the new residents with information relative to public transportation, off-street parking arrangements, and bicycle storage.

In conclusion, given the location of the proposed site, the availability of a strong public transportation accommodation and the available walkable and cyclable streets, this project and other similar projects should have little or no impact on the area traffic.

4

## TRAFFIC OPERATIONS

Measuring existing traffic volumes and projecting future traffic volumes quantify traffic flow within the study area. To assess the quality of traffic flow, intersection capacity analyses were performed to assess existing baseline conditions and for projected future design year (2029) conditions with and without development of the proposed residential project. Intersection capacity analyses demonstrate how well roadway facilities and their components serve the traffic demands placed upon them. This section includes potential on-site and off-site mitigation improvements should any be deemed necessary to minimize the impact of the proposed residential development on the area streets.

## Traffic operations measures

Level of Service (LOS) analysis that was performed for the residential project at 39 Lamartine Street was used for this study. The methodology used to determine the quality of traffic flow throughout the study area is based on the Highway Capacity Manual and its computer software.

The LOS term shows how well roadways, and their components process traffic placed upon them. Like a report card, LOSs are given letter designations from "A" to "F". LOS "A" represents the best operating conditions, while LOS "F" represents the worst. Typically, LOS "D" is considered acceptable during peak hour conditions, but LOS "E" may also be acceptable under some circumstances.

The LOS term is evaluated differently for unsignalized intersections. For an unsignalized intersection, the Highway Capacity analysis assumes that through traffic on major roadways is not affected by traffic on side streets (streets with lower volumes and/or ones under stop sign control). Therefore, a LOS term is typically calculated for the controlled movements (minor street approaches and major street left-turn movements). As described in the following paragraphs, capacity or LOS analyses were considered for year 2024 existing, year 2029 future no build, and year 2029 future build conditions for morning and evening peak hour periods at the above-mentioned intersections within the study area.

## **Existing Conditions**

Intersection capacity analyses were performed for the same five intersections as those for the 39 Lamartine Street project located within the study area during morning and evening peak traffic periods. These intersections are considered the only locations in the vicinity of the proposed site that may be affected by the traffic expected to be generated by the proposed residential development. All five intersections are unsignalized and are stop sign controlled.

The analysis concluded that LOS "A" is calculated for all controlled approaches of these intersections during both AM and PM peak periods and LOS "A" for the Intersection Capacity Utilization during both peak periods. A summary of the intersection analyses results for existing conditions is shown below in Table 5.

### **Future Conditions**

Capacity analyses for the future year peak hour traffic operations were performed for the year 2029 volumes during both peak periods with and without the proposed multifamily development project in place. A summary of the intersection analyses results for both future no-build and future build conditions is also shown below in Table 5.

As noted earlier in this report, based on the *massDOT* Traffic Volume and Classification data, area streets are included in group U4-U7 for the Growth Factor and Seasonal Factor. Based on roadways in group U4-U7, the yearly growth rate for this group of roadways is calculated at 2% per year. Therefore, an adjustment factor of 2% per year was used, and the collected TMCs were increased by 10% to reflect adjusted volumes for the future year 2029. Additionally, the Worcester Planning & Regulatory Division was consulted to identify potential nearby future developments that may have an impact on the intersections within the study area. Five projects were identified, and their locations were considered too far from the proposed development, thus it was determined that their traffic didn't reach any of the intersections within the study area. However, the traffic from a development at 39 Lamartine Street which includes 36 apartment units residential building with 1,581-sf first floor retail, also proposed by the applicant, was taken into consideration. The following Table 4 and Figure 6 show the trip generation and distribution for the proposed site at 39 Lamartine Street. The projected traffic volumes from this development were added to the year 2029 volumes to represent future no build conditions. Figure 7 shows the volumes for the future no-build conditions for the intersections in the study area.

Table 4 – Trip Generation and Distribution for 39 Lamartine Street 36 Apartments ITE Land Use Code 231

Rate-Trips/Unit Trips

Daily	%In	%Out	AM Pk	%In	%Out	PM Pk	%In	%Out
3.1*	50%	50%	0.2	39%	61%	0.28	44%	56%
117*	58	59	7	3	4	10	4	6

\*Estimated based on massDOT K factor of 0.09

1,581 sf Apparel Store ITE Land Use Code 876

Rate-Trips/1,000 sf Trips

I	Daily	%In	%Out	AM Pk	%In	%Out	PM Pk	%In	%Out
	12.4*	50%	50%	0.38	59%	41%	1.12	42%	58%
ı	20*	10	10	1	1	0	2	1	1

\*Estimated based on massDOT K factor of 0.09

**Grand Total Trips** 

137*	68	69	8	4	4	12	5	7
*Estimated b	ased on mass	DOT K factor of	of 0.09					

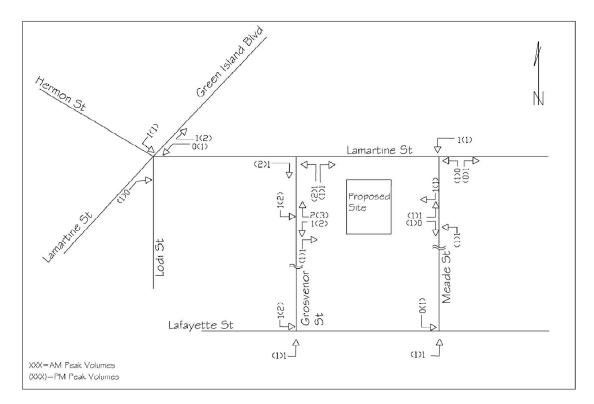
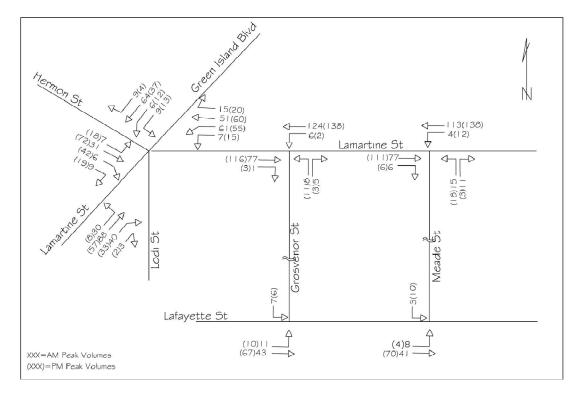


Figure 6 - Trip Generation and Distribution for 39 Lamartine Street

Figure 7 – Turning Movement Counts, Future No Build Conditions



Build traffic volumes were determined by projecting site-generated traffic volumes and distributing those volumes over the intersections within the study area, and finally, adding them to the future no-build conditions volumes. The following Figure 8 shows future build conditions traffic volumes for these intersections.

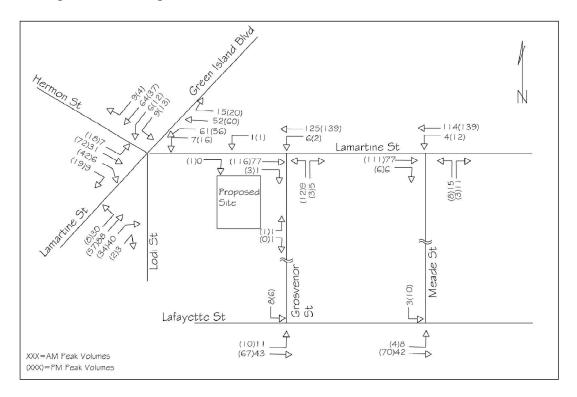


Figure 8 – Turning Movement Counts, Future Build Conditions

The intersection LOSs for the year 2029 no-build conditions were calculated for the controlled approaches of the intersections within the study area. It is expected the northbound approaches of the intersections of Lamartine with Meade Street and Grosvenor Street will be operating at LOS "B" during evening peak period. All other controlled intersection approaches will continue to operate at LOS "A". Finally, the Intersection Capacity Utilization will also remain at LOS "A" for all intersections.

To assess the potential traffic impact of the proposed development on these intersections, all traffic from the proposed residential development site at 10 Grosvenor Street was assigned to the approaches of these five intersections. The above Figures 2, 7 and 8 show the volumes at these intersections for both the morning and evening peak hours under existing, future no-build, and future build conditions.

The intersection analyses for the year 2029 build conditions were performed for all five intersections. The analysis revealed that under future build conditions, all controlled approaches of these intersections will continue to operate at the same level as those under future no-build conditions and the same Intersection Capacity Utilization LOS "A" as under no-build conditions.

Again, the above-mentioned LOS "B" for the northbound approaches of Meade Street and Grosvenor Street at their intersections with Lamartine Street under future no-build and future build conditions is indicative of little or no delays of traffic at these intersections and represents no impact associated with the proposed residential development project. A summary of intersection analyses for all five intersections under build conditions is provided below in Table 5. The computer printouts of the above-mentioned analysis are included in the Technical Appendix section of this report.

Table 5
Level Of Service Analysis Results Summary

Intersection	AM Peak	Existing		Future No	Build		Future Build	d	
Lamartine/Meade Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
/plume/Capacity	0.05	0.01	0.05	0.06	0.01	0.06	0.06	0.01	0.06
Approach Delay	0	0.5	9.5	0	0.7	9.6	0	0.7	9.6
OS			Α			Α			Α
nt Capacity Utilization LOS	Α			А			А		
	PM Peak								
	EB	WB	NB	EB	WB	NB	EB	WB	NB
/plume/Capacity	0.08	0.01	0.03	0.09	0.01	0.04	0.09	0.01	0.04
Approach Delay	0	0.8	10	0	0.8	10.4	0	0.8	10.4
_OS			Α			В			В
nt Capacity Utilization LOS	А	ı	I 6	Α		le:	А	1	
	AM Peak								
Lamartine/Grosvenor Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
Vplume/Capacity	0.04	0.01	0.02	0.05	0.01	0.03	0.05	0.01	0.03
Approach Delay	0	0.7	9.3	0	0.7	9.5	0	0.7	9.5
LOS	3000	00000000	Α		220000	A	1902	4000	Α
Int Capacity Utilization LOS	Α	-		А			Α		
, , , , , , , , , , , , , , , , , , , ,				3.					
	PM Peak		Ne		1110	110		1110	
Lamartine/Grosvenor Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
Vplume/Capacity	0.08	0	0.02	0.09	0	0.03	0.09	0	0.03
Approach Delay	0	0.2	10	0	0.2	10.2	0	0.2	10.2
LOS			Α			В			В
Int Capacity Utilization LOS	Α			Α			А		
ntersection	AM Peak	Existing		Future No I	Build		Future Build	1	
Lafayette/Meade Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
			8925177	6.0000		592.70	0.000		
/plume/Capacity	0	0	0.01	0.01	0	0.01	0.01	0	106510783
V 155	0	0	0.01 8.9	0.01	0	0.01 8.9	0.01 1.2	0	0.01
Approach Delay	1	0	0.0 <b>1</b> 8.9	0.01 1.2	0	0.01 8.9 A	0.01 1.2		106510783
Approach Delay LOS	1	-	8.9	1.2		8.9	1.2		0.01 8.9
Approach Delay LOS	-	-	8.9			8.9			0.01 8.9
Approach Delay LOS	A A PM Peak	0	8.9 A	1.2 A	0	8.9 A	1.2 A	0	0.01 8.9 A
Approach Delay LOS nt Capacity Utilization LOS	A PM Peak	0 WB	8.9 A	1.2 A	0 WB	A A NB	1.2 A	0 WB	0.01 8.9 A
Approach Delay LOS Int Capacity Utilization LOS	A  PM Peak  EB  0	0 WB 0	8.9 A NB 0.02	1.2 A EB 0	WB 0	8.9 A NB 0.02	1.2 A EB 0	WB 0	0.01 8.9 A NB 0.02
Approach Delay LOS Int Capacity Utilization LOS  //plume/Capacity  Approach Delay	A PM Peak	0 WB	8.9 A NB 0.02	1.2 A	0 WB	8.9 A NB 0.02 9.1	1.2 A	0 WB	0.01 8.9 A NB 0.02 9.1
Approach Delay LOS Int Capacity Utilization LOS  /plume/Capacity  Approach Delay LOS	A  PM Peak  EB  0  0.3	0 WB 0	8.9 A NB 0.02	1.2 A EB 0 0.4	WB 0	8.9 A NB 0.02	1.2 A EB 0 0.4	WB 0	0.01 8.9 A NB 0.02
Approach Delay LOS Int Capacity Utilization LOS  /plume/Capacity  Approach Delay LOS	A  PM Peak  EB  0	0 WB 0	8.9 A NB 0.02	1.2 A EB 0	WB 0	8.9 A NB 0.02 9.1	1.2 A EB 0	WB 0	0.01 8.9 A NB 0.02 9.1
Approach Delay LOS Int Capacity Utilization LOS  Vplume/Capacity  Approach Delay LOS	A  PM Peak  EB  0  0.3	0 WB 0	8.9 A NB 0.02	1.2 A EB 0 0.4	WB 0	8.9 A NB 0.02 9.1	1.2 A EB 0 0.4	WB 0	0.01 8.9 A NB 0.02 9.1
Approach Delay LOS Int Capacity Utilization LOS  Vplume/Capacity  Approach Delay LOS Int Capacity Utilization LOS	A  PM Peak  EB  0  0.3	0 WB 0	8.9 A NB 0.02	1.2 A EB 0 0.4	WB 0	8.9 A NB 0.02 9.1	1.2 A EB 0 0.4	WB 0	0.01 8.9 A NB 0.02 9.1
Approach Delay LOS Int Capacity Utilization LOS  Approach Delay LOS Int Capacity Utilization LOS  Approach Delay LOS Int Capacity Utilization LOS	A  PM Peak  EB  0  0.3  A  AM Peak	0 WB 0 0	8.9 A NB 0.02 9 A	1.2 A EB 0 0.4	0 WB 0 0	8.9 A NB 0.02 9.1 A	1.2 A  EB 0 0.4 A	0 WB 0 0	0.01 8.9 A NB 0.02 9.1 A
Approach Delay LOS Int Capacity Utilization LOS  Approach Delay LOS Int Capacity Utilization LOS  Approach Delay LOS Int Capacity Utilization LOS  Lafayette/Grosvenor Sts  Application LOS	A  PM Peak  EB  0  0.3  A  AM Peak  EB	0 WB 0 0	8.9 A NB 0.02 9 A	1.2 A EB 0 0.4 A	0 WB 0 0	8.9 A NB 0.02 9.1 A	1.2 A  EB  0  0.4  A	0 WB 0 0	0.01 8.9 A NB 0.02 9.1 A
Approach Delay LOS Int Capacity Utilization LOS Int Capacity Utilization LOS Int Capacity Int Capacity Int Capacity Int Capacity Utilization LOS Int Capacity Utilization LOS Int Capacity Utilization LOS Int Capacity Int	A  PM Peak  EB  0  0.3  A  AM Peak  EB  0.01	0 WB 0 0 WB 0	8.9 A NB 0.02 9 A NB 0.01	A EB 0 0.4 A EB 0.01	0 WB 0 0	8.9 A NB 0.02 9.1 A	1.2 A EB 0 0.4 A	0 WB 0 0 WB 0	0.01 8.9 A NB 0.02 9.1 A
Approach Delay  Approach Delay  Approach Delay  Approach Delay  OS  afayette/Grosvenor Sts  Approach Delay  Lapayette/Grosvenor Delay  Approach Delay  Approach Delay  Approach Delay  Approach Delay  Approach Delay	A  PM Peak  EB  0  0.3  A  AM Peak  EB  0.01	0 WB 0 0 WB 0	8.9 A NB 0.02 9 A NB 0.01 8.9	A EB 0 0.4 A EB 0.01	0 WB 0 0	NB 0.02 9.1 A	1.2 A EB 0 0.4 A	0 WB 0 0 WB 0	0.01 8.9 A NB 0.02 9.1 A NB 0.01
Approach Delay LOS Int Capacity Utilization LOS  Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS  Lafayette/Grosvenor Sts Vplume/Capacity Approach Delay LOS	1 A PM Peak EB 0 0.3 A AM Peak EB 0.01 1.4	0 WB 0 0 WB 0	8.9 A NB 0.02 9 A NB 0.01 8.9	A EB 0 0.4 A EB 0.01 1.5	0 WB 0 0	NB 0.02 9.1 A	1.2  A  EB  0  0.4  A  EB  0  1.5	0 WB 0 0 WB 0	0.01 8.9 A NB 0.02 9.1 A NB 0.01
Approach Delay LOS Int Capacity Utilization LOS  Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS  Lafayette/Grosvenor Sts Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS	1 A PM Peak EB 0 0.3 A AM Peak EB 0.01 1.4 A PM Peak	0 WB 0 0 0 WB 0 0 0	8.9 A NB 0.02 9 A NB 0.01 8.9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5	0 WB 0 0 0 WB 0 0 0	NB 0.02 9.1 A NB 0.01 9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5	WB 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.01 8.9 A NB 0.02 9.1 A NB 0.01 9
Approach Delay LOS  Int Capacity Utilization LOS  Vplume/Capacity Approach Delay LOS  Int Capacity Utilization LOS  Lafayette/Grosvenor Sts Vplume/Capacity Approach Delay LOS  Int Capacity Utilization LOS  Lafayette/Grosvenor Sts  Vplume/Capacity Approach Delay LOS  Lamartine/Grosvenor Sts	1 A PM Peak EB 0 0.3 A A AM Peak EB 0.01 1.4 A PM Peak EB	0 WB 0 0 0 WB WB WB	8.9 A NB 0.02 9 A NB 0.01 8.9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5  A	0 WB 0 0 0 WB WB WB	NB 0.02 9.1 A NB 0.01 9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5  A	WB 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.01 8.9 A NB 0.02 9.1 A NB 0.01 9 A
Approach Delay LOS Int Capacity Utilization LOS Lafayette/Grosvenor Sts Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS Lamartine/Grosvenor Sts Vplume/Capacity	1 A PM Peak EB 0 0.3 A AM Peak EB 0.01 A A PM Peak EB 0.01	0 WB 0 0 0 0	8.9 A NB 0.02 9 A NB 0.01 8.9 A	A EB 0 0.4 A EB 0.01 1.5 A EB 0.01	0 WB 0 0 0 0	NB 0.02 9.1 A NB 0.01 9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5  A	WB 0 0 0 0 WB 0 0 0 0 0 0 0 0 0 0 0 0 0	0.01 8.9 A NB 0.02 9.1 A NB 0.01 9 A
Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS  Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS  Lafayette/Grosvenor Sts Vplume/Capacity Approach Delay LOS Int Capacity Utilization LOS  Lamartine/Capacity Approach Delay LOS Lamartine/Grosvenor Sts Vplume/Capacity Approach Delay LOS Lamartine/Grosvenor Sts Vplume/Capacity Approach Delay LOS	1 A PM Peak EB 0 0.3 A A AM Peak EB 0.01 1.4 A PM Peak EB	0 WB 0 0 0 WB WB WB	8.9 A NB 0.02 9 A NB 0.01 8.9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5  A	0 WB 0 0 0 WB WB WB	NB 0.02 9.1 A NB 0.01 9 A	1.2  A  EB  0  0.4  A  EB  0.01  1.5  A	WB 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.01 8.9 A NB 0.02 9.1 A NB 0.01 9 A

	AM Peak	Existing			Future No	Build			Future Buil	ld		
Lamartine/Hermon/Green Island	EB	WB	NB	SB	EB	WB	NB	SB	EB	WB	NB	SB
Vplume/Capacity	0.21	0.15	0.18	0.07	0.24	0.17	0.2	0.09	0.24	0.17	0.2	0.09
Approach Delay	8.4	8.1	8.1	7.9	8.7	8.3	8.4	8.1	8.7	8.3	8.4	8.1
LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α
Int Capacity Utilization LOS	Α				Α				Α			
	PM Peak											
Lamartine/Hermon/Green Island	EB	WB	NB	SB	EB	WB	NB	SB	EB	WB	NB	SB
Vplume/Capacity	0.14	0.13	0.2	0.23	0.16	0.14	0.23	0.26	0.16	0.15	0.23	0.26
Approach Delay	8.2	8.1	8.2	8.7	8.5	8.3	8.5	9	8.5	8.3	8.5	9
LOS	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α	Α
Int Capacity Utilization LOS	Α				Α				Α			

# 5 FINDINGS

This traffic study was conducted to evaluate the potential traffic impacts associated with the proposed residential development site located on the west side of Grosvenor Street in the city of Worcester, Massachusetts. This study includes the evaluation of five intersections which are in close proximity to the site and were also evaluated for the 39 Lamartine Street mixed use project which are likely to be impacted by the proposed residential development project. Evaluation of these intersections, which were identified by consultation with Worcester DTM, and which is intended to identify capacity constraints was performed for existing, future no-build, and future build conditions. Future analyses have determined that the site-generated traffic volumes are not significant, and they can certainly be accommodated with the existing roadways, their intersections, and the site driveways off Lamartine Street and Grosvenor Street. These analyses demonstrated that with the additional traffic volumes associated with the proposed development, the Intersection Capacity Utilization LOSs will also stay the same as those of future no build conditions LOS "A". The analysis also concluded that the approaches of all five intersections are and will continue to operate at LOS "A" except the northbound approaches of Meade and Grosvenor Streets at Lamartine Street which will be operating at LOS "B" under future conditions during afternoon peak period with or without the proposed residential development project.

## Conclusion & Recommendations

It is concluded that of the five intersections evaluated, the intersection of Lafayette Street and Meade Street which has had two accidents over a five-year period has experienced an accident rate greater than average for unsignalized intersections, and therefore, a remedy is warranted at this intersection. It is believed that the on-street parking on the north side of Lafayette Street at Meade Street may have contributed to these two accidents. The Intersection of Lafayette Street, Lodi Street, Hermon Street and Green Island Boulevard had an accident rate of 0.35 which is significantly lower than average for unsignalized intersections. The remaining three intersections had no accidents reported during this five-year period.

The available sight distances for the proposed Grosvenor Street exit-only driveway will allow motorists to safely exit the site in order to enter the flow of traffic on Grosvenor Street.

The volumes of traffic associated with the proposed residential development are considered inconsequential, and therefore, the surrounding streets have ample capacity to safely serve the anticipated additional traffic. The level of service evaluation presented above is an indicator of the quality of traffic flow through the area. This evaluation indicates that no impact is expected from the proposed development and the LOS will not fall below "A" for all approaches of the intersections studied except for the northbound approaches of Meade and Grosvenor Streets at Lamartine Street which will be operating at LOS "B" during PM peak period under future conditions with or without the proposed development project. Also, since there is little or no opportunity for landscaping along the property lines on Grosvenor Street, the site distance to the

left of the proposed exit-only driveway is not expected to be impacted. To maintain optimum safety and efficiency, the following recommendations, most of which are included in the Transportation Demand Management program of this report, should be considered.

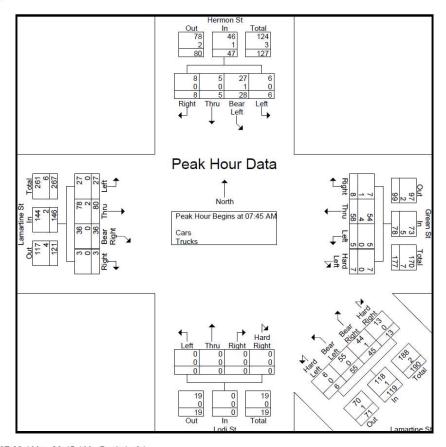
- It is recommended that the city of Worcester reinstall the missing stop signs mentioned in this report.
- To improve intersection sight distance and to minimize the potential for angle-type accidents, it is recommended that on-street parking on the north side of Lafayette Street be prohibited from the Meade Street intersection westerly for 50 feet.
- Information regarding public transportation services should be made available to residents and include maps, schedules and fare information.
- Assign a transportation coordinator or superintendent of the building for the proposed project, who may also have other responsibilities, to coordinate the TDM program.
- A "welcome packet" should be provided to new residents comprising the name and contact information for the transportation coordinator/building superintendent and describing available public transportation services, bicycle and walking alternatives, and other amenities and commuting options.
- Consider providing secure bicycle parking consisting of both weather-protected bicycle parking and exterior bicycle racks.
- Consult with the WRTA to discuss options to establish transit service to the streets near the project site such as Green Island Boulevard and Lamartine Street including potential installation of a dynamic sign displaying live information relative to WRTA bus routes.
- Work-at-home accommodation should be included within the Project and may take the form of a meeting space or a business office in the common or lobby area of the building.

Technical Appendix

### Accurate Counts

978-664-2565

N/S Street: Hermon St / Lodi St E/W Street: Green St / Lamartine St City/State: Worcester, MA Weather: Clear / Cloudy File Name : 20260001 Site Code : 20260001 Start Date : 7/17/2024 Page No : 2

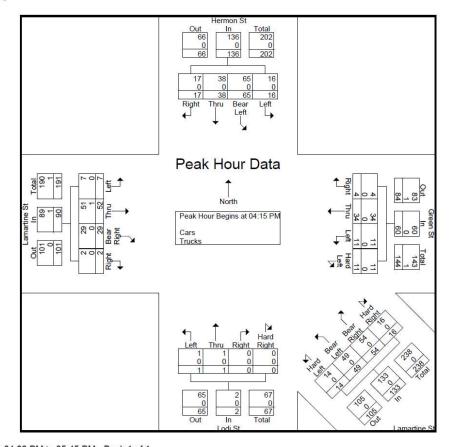


Peak Hour	Analysis From	07:00 AM to	08:45 AM - Peak 1 of 1

Peak Hour	for E	ach A	pproa	ich Be	egins a	t:																			
_	08:00 AM	E .				07:45 AM					07:45 AM					07:00 AM					07:45 AM				
+0 mins.	2	10	0	3	15	1	0	17	2	20	1	12	13	4	30	0	0	0	0	0	8	19	12	0	39
+15 mins.	1	5	0	1	7	3	0	13	3	19	0	10	12	1	23	0	0	0	0	0	3	20	6	0	29
+30 mins.	3	8	3	2	16	0	4	14	3	21	1	19	11	4	35	0	0	0	0	0	9	21	9	3	42
+45 mins.	3	8	1	2	14	3	1	14	0	18	4	14	9	4	31	0	0	0	0	0	7	20	9	0	36
Total Volume	9	31	4	8	52	7	5	58	8	78	6	55	45	13	119	0	0	0	0	0	27	80	36	3	146
% App. Total	17.3	59.6	7.7	15.4		9	6.4	74.4	10.3		5	46.2	37.8	10.9		0	0	0	0		18.5	54.8	24.7	2.1	
PHF	.750	.775	.333	.667	.813	.583	.313	.853	.667	.929	.375	.724	.865	.813	.850	.000	.000	.000	.000	.000	.750	.952	.750	.250	.869
Cars	9	30	4	8	51	7	5	54	7	73	6	55	44	13	118	0	0	0	0	0	27	78	36	3	144
% Cars	10	96.	10 0	10	98.1	10	10	93. 1	87. 5	93.6	10	10	97. 8	10	99.2	0	0	0	0	0	10	97. 5	10	10	98.6
Trucks	0	1	0	0	1	0	0	4	1	5	0	0	1	0	1	0	0	0	0	0	0	2	0	0	2
% Trucks	0	3.2	0	0	1.9	0	0	6.9	12. 5	6.4	0	0	2.2	0	0.8	0	0	0	0	0	0	2.5	0	0	1.4

N/S Street: Hermon St / Lodi St E/W Street: Green St / Lamartine St City/State: Worcester, MA Weather: Clear / Cloudy

File Name : 20260001 Site Code : 20260001 Start Date : 7/17/2024 Page No : 2

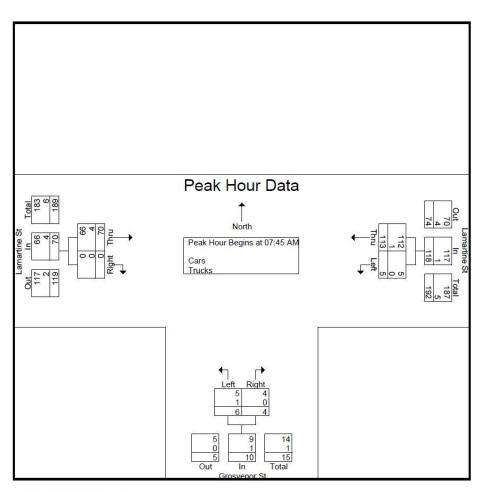


Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Fach Approach Regime at:

Peak Hour	for E	ach A	pproa	ach Be	egins a	it:																			
	04:15 PN					04:00 PN	1				04:15 PN	1				04:15 PM	6				04:00 PM	1			
+0 mins.	5	13	6	6	30	0	2	15	2	19	4	8	16	4	32	0	0	0	0	0	1	14	9	2	26
+15 mins.	2	16	9	3	30	3	4	7	1	15	3	16	9	3	31	1	0	0	0	1	1	15	7	1	24
+30 mins.	1	16	9	5	31	4	4	8	2	18	3	8	13	5	29	0	0	0	0	0	2	15	5	1	23
+45 mins.	8	20	14	3	45	1	0	7	0	8	4	17	16	4	41	0	1	0	0	1	3	10	6	0	19
Total Volume	16	65	38	17	136	8	10	37	5	60	14	49	54	16	133	1	1	0	0	2	7	54	27	4	92
% App. Total	11.8	47.8	27.9	12.5		13.3	16.7	61.7	8.3		10.5	36.8	40.6	12		50	50	0	0		7.6	58.7	29.3	4.3	
PHF	.500	.813	.679	.708	.756	.500	.625	.617	.625	.789	.875	.721	.844	.800	.811	.250	.250	.000	.000	.500	.583	.900	.750	.500	.885
Cars	16	65	38	17	136	8	10	37	5	60	14	49	54	16	133	1	1	0	0	2	7	52	26	4	89
% Cars	10	10	10	10	100	10	10	10	10	100	10	10	10	10	100	10	10	0	0	100	10	96. 3	96. 3	10	96.7
Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	1	0	3
% Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	37	37	0	33

N/S Street : Lamartine Street E/W Street : Grosvenor Street City/State : Worcester, MA Weather : Clear / Cloudy

File Name : 20260002 Site Code : 20260002 Start Date : 7/17/2024 Page No : 2



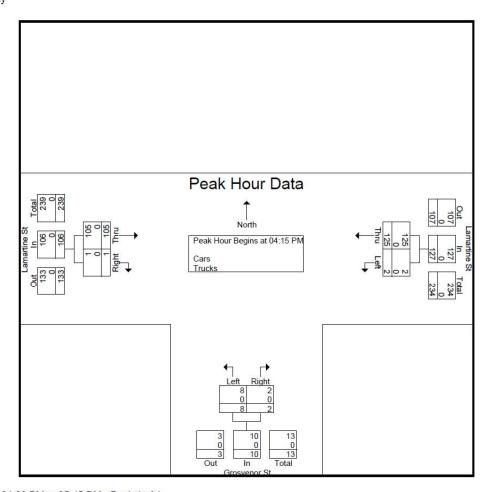
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak	Hour	for	Each	App	roach	ı E	egin	s at:

	07:45 AM			08:00 AM			07:45 AM		
+0 mins.	0	30	30	1	1	2	18	0	18
+15 mins.	1	21	22	2	1	3	19	0	19
+30 mins.	3	33	36	3	0	3	14	0	14
+45 mins.	1	29	30	3	2	5	19	0	19
Total Volume	5	113	118	9	4	13	70	0	70
% App. Total	4.2	95.8		69.2	30.8	3	100	0	
PHF	.417	.856	.819	.750	.500	.650	.921	.000	.921
Cars	5	112	117	8	4	12	66	0	66
% Cars	100	99.1	99.2	88.9	100	92.3	94.3	0	94.3
Trucks	0	1	1	1	0	1	4	0	4
% Trucks	0	0.9	0.8	11.1	0	7.7	5.7	0	5.7

N/S Street : Lamartine Street E/W Street : Grosvenor Street City/State : Worcester, MA Weather : Clear / Cloudy

File Name : 20260002 Site Code : 20260002 Start Date : 7/17/2024 Page No : 2

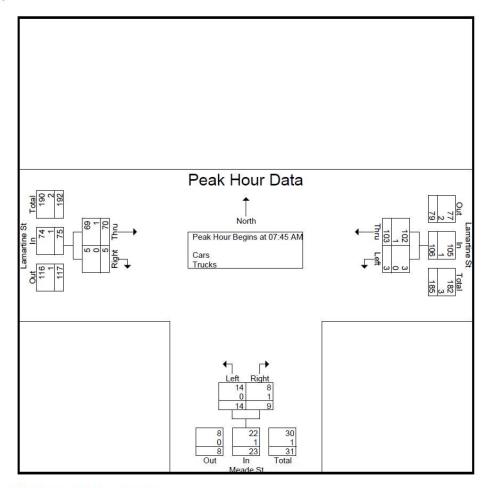


Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour	for Each A	pproach	Begins at:
-----------	------------	---------	------------

	04:15 PM			04:30 PM			04:15 PM		
+0 mins.	0	31	31	4	1	5	24	0	24
+15 mins.	0	28	28	1	0	1	25	0	25
+30 mins.	1	29	30	2	1	3	23	0	23
+45 mins.	1	37	38	3	0	3	33	1	34
Total Volume	2	125	127	10	2	12	105	1	106
% App. Total	1.6	98.4	20000-0	83.3	16.7	-	99.1	0.9	30.750.000
PHF	.500	.845	.836	.625	.500	.600	.795	.250	.779
Cars	2	125	127	10	2	12	105	1	106
% Cars	100	100	100	100	100	100	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0

N/S Street : Meade Street E/W Street : Lamartine Street City/State : Worcester, MA Weather : Clear / Cloudy File Name : 20260003 Site Code : 20260003 Start Date : 7/17/2024 Page No : 2



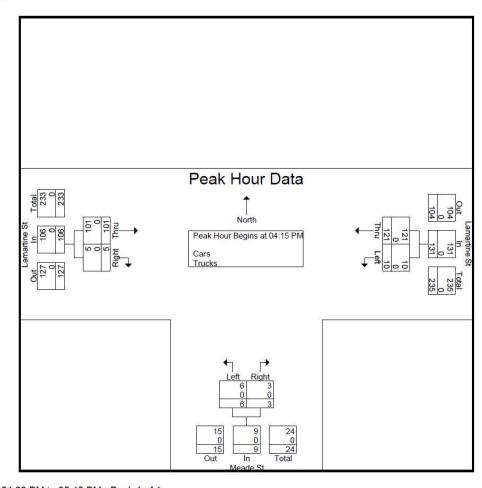
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Dook Hour for	Each Approach	Dogina at:
reak noul lol	Each Approach	Dedins at.

	07:45 AM		13	08:00 AM			07:45 AM		
+0 mins.	0	27	27	2	2	4	19	1	20
+15 mins.	1	21	22	7	1	8	20	1	21
+30 mins.	0	29	29	4	5	9	14	1	15
+45 mins.	2	26	28	2	8	10	17	2	19
Total Volume	3	103	106	15	16	31	70	5	75
% App. Total	2.8	97.2	- 3	48.4	51.6		93.3	6.7	
PHF	.375	.888	.914	.536	.500	.775	.875	.625	.893
Cars	3	102	105	15	15	30	69	5	74
% Cars	100	99	99.1	100	93.8	96.8	98.6	100	98.7
Trucks	0	1	1	0	1	1	1	0	1
% Trucks	0	1	0.9	0	6.2	3.2	1.4	0	1.3

N/S Street : Meade Street E/W Street : Lamartine Street City/State : Worcester, MA Weather : Clear / Cloudy

File Name : 20260003 Site Code : 20260003 Start Date : 7/17/2024 Page No : 2



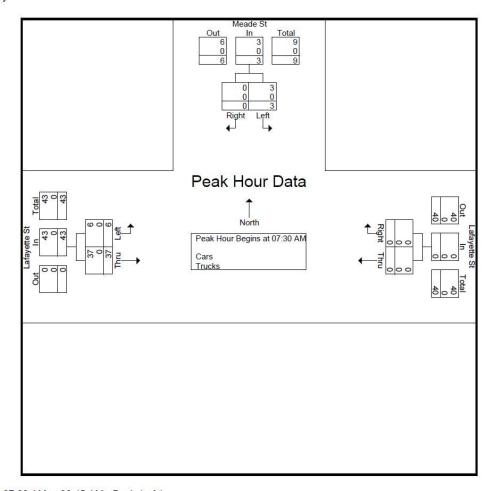
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PM			05:00 PM			04:15 PM		
+0 mins.	3	32	35	4	1	5	23	1	24
+15 mins.	4	31	35	0	0	0	25	1	26
+30 mins.	2	28	30	1	1	2	20	2	22
+45 mins.	3	29	32	1	3	4	33	1	34
Total Volume	12	120	132	6	5	11	101	5	106
% App. Total	9.1	90.9		54.5	45.5		95.3	4.7	
PHF	.750	.938	.943	.375	.417	.550	.765	.625	.779
Cars	12	120	132	6	5	11	101	5	106
% Cars	100	100	100	100	100	100	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0

N/S Street : Meade Street E/W Street : Lafayette Street City/State : Worcester, MA Weather : Clear / Cloudy

File Name : 20260004 Site Code : 20260004 Start Date : 7/17/2024

Page No : 2



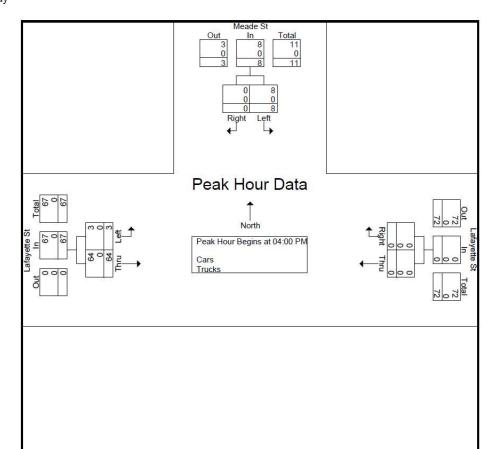
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Appl	roach Begins at:

	07:00 AM			07:00 AM			08:00 AM		
+0 mins.	1	0	1	0	0	0	3	9	12
+15 mins.	0	0	0	0	0	0	3	10	13
+30 mins.	3	0	3	0	0	0	2	5	7
+45 mins.	0	0	0	0	0	0	1	11	12
Total Volume	4	0	4	0	0	0	9	35	44
% App. Total	100	0		0	0		20.5	79.5	
PHF	.333	.000	.333	.000	.000	.000	.750	.795	.846
Cars	4	0	4	0	0	0	8	35	43
% Cars	100	0	100	0	0	0	88.9	100	97.7
Trucks	0	0	0	0	0	0	1	0	1
% Trucks	0	0	0	0	0	0	11.1	0	2.3

N/S Street : Meade Street E/W Street : Lafayette Street City/State : Worcester, MA Weather : Clear / Cloudy

File Name : 20260004 Site Code : 20260004 Start Date : 7/17/2024 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for	Each	Appr	oach	Begins a	at:
				The state of the s	

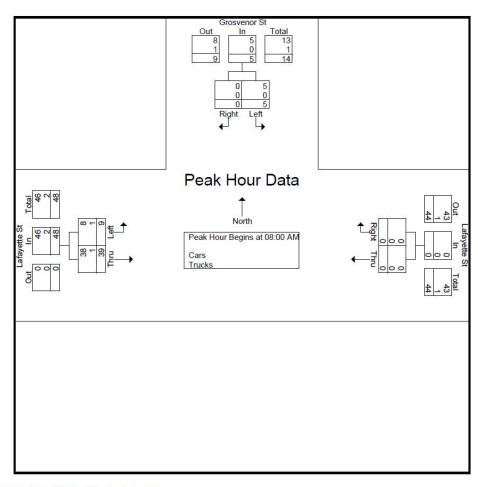
	04:00 PM	201		04:00 PM			04:00 PM		
+0 mins.	2	0	2	0	0	0	1	13	14
+15 mins.	0	0	0	0	0	0	1	15	16
+30 mins.	4	0	4	0	0	0	0	15	15
+45 mins.	2	0	2	0	0	0	1	21	22
Total Volume	8	0	8	0	0	0	3	64	67
% App. Total	100	0		0	0		4.5	95.5	
PHF	.500	.000	.500	.000	.000	.000	.750	.762	.761
Cars	8	0	8	0	0	0	3	64	67
% Cars	100	0	100	0	0	0	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0

## **Accurate Counts**

978-664-2565

N/S Street : Grosvenor Street E/W Street : Lafayette Street City/State : Worcester, MA Weather : Clear / Cloudy

File Name : 20260005 Site Code : 20260005 Start Date : 7/17/2024 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

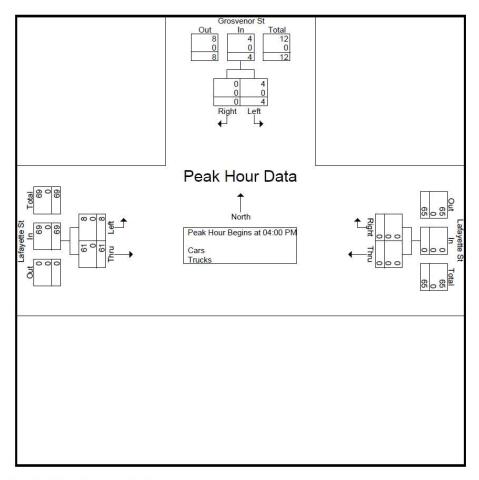
Peak Hou	ur for Each	Approach	Begins at:

	08:00 AM			07:00 AM			08:00 AM		
+0 mins.	1	0	1	0	0	0	2	10	12
+15 mins.	2	0	2	0	0	0	3	13	16
+30 mins.	1	0	1	0	0	0	3	5	8
+45 mins.	1	0	1	0	0	0	1	11	12
Total Volume	5	0	5	0	0	0	9	39	48
% App. Total	100	0		0	0		18.8	81.2	
PHF	.625	.000	.625	.000	.000	.000	.750	.750	.750
Cars	5	0	5	0	0	0	8	38	46
% Cars	100	0	100	0	0	0	88.9	97.4	95.8
Trucks	0	0	0	0	0	0	1	1	2
% Trucks	0	0	0	0	0	0	11.1	2.6	4.2

## **Accurate Counts**

978-664-2565

N/S Street: Grosvenor Street E/W Street: Lafayette Street City/State: Worcester, MA Weather: Clear / Cloudy File Name : 20260005 Site Code : 20260005 Start Date : 7/17/2024 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

	05:00 PM			04:00 PM			04:00 PM		
+0 mins.	1	0	1	0	0	0	3	14	17
+15 mins.	1	0	1	0	0	0	0	16	16
+30 mins.	1	0	1	0	0	0	4	12	16
+45 mins.	3	0	3	0	0	0	1	19	20
Total Volume	6	0	6	0	0	0	8	61	69
% App. Total	100	0		0	0		11.6	88.4	
PHF	.500	.000	.500	.000	.000	.000	.500	.803	.863
Cars	6	0	6	0	0	0	8	61	69
% Cars	100	0	100	0	0	0	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0

Accurate Counts 978-664-2565

7/15/2024	Monday		Tuesday		Wednesday	ay	Thursday	٨	Friday	1	Saturday	ЭУ	Sunday	ıy	Week Average	age
Time	SB, NB	3.	SB, N	18	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB,	SB,	NB.
12:00 AM	*	*	*	*	14	8	14	9	*	*	*	*	*	*	14	7
1:00	*	*	*	*	11	10	9	6	*	*	*	*	*	*	8	10
2:00	*	*	*	*	8	2	6	2	*	*	*	*	*	*	8	4
3.00	*	*	*	*	6	3	00	5	*	*	*	*	*	*	9	4
4:00	*	*		*	80	10	9	11	*	*	٠	*	٠	*	7	10
5:00	*	*	*	*	20	26	19	22	¥	*	*	*	*	*	20	24
6:00	*	*	*	*	41	32	36	50	*	*	*	*	*	*	30	33
7:00	*	*	*	*	48	45	40	44	*	*	*	*	*	*	44	44
8:00	*	*	*	*	53	70	44	69	*	*	٠	*	*	*	48	64
9:00	*	*	*	*	09	22	47	48	*	*	*	*	٠	*	54	52
10:00	*	*	*	*	34	29	69	33	*	*	*	*	*	*	52	31
11:00	*	*	*	*	64	49	99	44	*	*	*	*	*	*	65	46
12:00 PM	*	*	*	*	11	45	75	57	*	*	*	*	*	*	9/	48
1:00	*	*	*	*	79	20	82	70	*	*	٠	*	*	*	80	9
2:00	*	*	*	*	84	59	16	64	*	*	*	*	٠	*	06	62
3:00	*	*	*	٠	120	22	109	59	*	*	*	*	*	*	114	58
4:00	*	*	*	*	114	09	147	74	¥0	*	*	*	*	*	130	19
5:00	*	*	*	*	125	58	123	99	*	*	*	*	*	*	124	62
6:00	*	*	*	*	70	37	78	39	*	*	*	*	٠	*	74	38
7:00	*	*		*	22	27	73	33	*	*	*	*	*	*	65	30
8:00	*	*	*	٠	45	12	48	37	*	*	*	*	*	*	46	24
00:6	*	*	*	*	56	30	46	44	¥	*	*	*	*	*	51	37
10:00	*	*	*	*	23	15	32	18	×	*	×	*	*	*	28	16
11:00	*	*	*	*	18	80	23	19	*	*	*	*	*	*	20	14
Total	0	0	0	0	1232	802	1297	891	0	0	0	0	0	0	1262	845
Day	0	ā i	0	F (	2034		2188	55	0	90 1	0	88 8	0	· 67 · 8	2107	
AM Peak					11:00	8:00	10:00	8:00							11:00	8:00
Volume					64	70	69	59							65	64
PM Peak					2:00	4	4:00	4:00							4:00	4:00
Volume					125	09	147	74							130	67

ocation: Hermon Street Site Code: 20260001

Location: Hermon Street Location: North of Green Street City/State: Worcester, MA Direction: Combined

7/18/2024	0-3	> 3 - 6	> 6 - 9	> 9 - 12	> 12 - 15	> 15 - 18	> 18 -	> 21 - 24	> 24 - 27	> 27 - 30	> 30 -	> 33 - 36	> 36 -	> 39	
Time	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	Total
12:00 AM	0	0	1	1	1	6	1	6	3	1	0	0		0	20
1:00	0	0	0	3	2	4	2	2	1	1	0	0	0	0	15
2:00	0	0	0	0	3	4	2	2	0	0	0	0	0	0	11
3:00	0	0	0	1	6	0	2	3	0	0	1	0	0	0	13
4:00	0	0	0	0	2	5	4	3	2	1	0	0	0	0	17
5:00	0	0	0	0	1	9	15	11	3	1	1	0	0	0	41
6:00	0	0	4	2	5	15	11	16	12	3	1	1	0	0	70
7:00	0	0	7	3	2	4	18	22	20	8	0	0	0	0	84
8:00	0	0	7	2	6	10	24	21	20	11	2	0	0	0	103
9:00	0	0	5	3	3	9	19	35	11	5	3	2	0	0	95
10:00	0	0	2	5	7	15	27	28	13	1	1	3	0	0	102
11:00	0	0	1	7	12	14	35	18	13	5	3	2	0	0	110
12:00 PM	0	0	0	3	8	18	34	29	23	8	1	2	0	0	126
1:00	0	0	3	1	4	17	40	37	27	16	5	1	0	1	152
2:00	0	0	4	11	12	18	44	36	24	9	3	0	0	0	161
3:00	0	0	3	7	14	16	40	45	25	14	4	0	0	0	168
4:00	0	0	5	8	16	14	43	60	53	15	5	1	1	0	221
5:00	0	0	4	6	12	20	27	53	38	18	6	2	2	1	189
6:00	0	0	0	0	2	12	26	36	25	8	5	2	1	0	117
7:00	0	0	1	0	3	4	22	21	34	16	4	0	1	0	106
8:00	0	0	0	0	5	2	14	31	19	9	3	0	2	0	85
9:00	0	0	2	2	1	7	24	29	17	6	1	0	1	0	90
10:00	0	0	0	0	5	8	15	8	9	3	2	0	0	0	50
11:00	0	0	1	0	4	5	13	7	6	5	1	0	0	0	42
Total	0	0	50	65	136	236	502	559	398	164	52	16	8	2	2188
		P	ercentile	15th	50th	85th	95th								
			Speed	15	20	25	28								
	Mean	Speed (A	(verage)	21.7											
	10 N	MPH Pac	e Speed	18-27											
			in Pace	1484											
		Percent	in Pace	71.0%											
		umber >		1199											
		ercent >	21 MPH	54.8%											
Grand Total	0	0	120	118	279	515	990	988	729	328	110	29	10	6	4222
Stats		P	ercentile	15th	50th	85th	95th								
			Speed	15	20	25	28								
		Speed (A		21.5											
	10 N	MPH Pac		18-27											
		Number		2866											
		Percent	in Pace	69.0%											
	Sec.		THE STATE	4 10 10 12 12											
		umber > ercent >		2200 52.1%											

Accurate Counts 978-664-2565

7/15/2024	Monday		Tuesday	ý	Wednesday	sday	Thursday	ay	Friday		Saturday	,	Sunday	1	Week Average	rage
Time	WB, EB,		WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB,	EB,	WB.	EB,
12:00 AM	*	*	*	*	24	19	19	12	*	*	*	*	*	*	22	16
1.00	*	*	*	*	20	8	19	15	*)	*	٠	*	*	*	20	12
2:00	*	*	*	*	15	12	9	9	*	*	٠	*	*	*	10	6
3:00	*	*	*	*	5	10	12	7	*	*	*	*	*	*	8	8
4.00	*	*	*	*	12	20	21	18	*	*		3 <b>*</b>	*	*	16	19
5:00	*	*	•	*	37	43	30	40	*	*	*	*	*	*	34	42
6:00	*	*	*	*	56	75	51	78	*	*	*	*	*	*	54	91
7:00	*	*	*	*	80	127	83	96	*	*	*	*	*	*	82	111
8:00	*	*	*	*	114	132	104	128	*	*		*	*	*	109	130
9:00	*	*	*	*	85	101	61	92	*	*	*	*	*	*	82	96
10:00	•	*	*	*	100	85	92	11	*	*	*	*	*	*	96	81
11:00	*	*	*	*	98	82	108	93	*	*	٠	*	*	*	16	88
12:00 PM	*	*	*	*	66	106	121	86	*	*	*	*	*	*	110	102
1.00	*	*	*	*	110	06	125	118	*	*	*	*	*	*	118	104
2.00		*	*	*	118	102	109	89	*	*	*	*	*	*	114	96
3:00		*	٠	*	104	91	121	83	*	*	*	*	*	*	112	87
4:00	*	*	*	*	103	98	125	124	*	*	*	*	*	*	114	105
5:00	*	*	*	*	102	81	26	131	*	*	*	*	*	*	100	106
6:00	*	*	*	*	98	8	6/	74	*	*	*	:*		*	82	79
7.00		*	*	*	83	49	93	72	*	*	*	*	*	*	88	89
8:00	*	*	٠	*	61	46	69	65	*	*	•	*	*	*	65	99
00.6	*	*	*	*	54	53	29	75	٠	*	٠	*	*	*	09	64
10:00	*	*	*	*	39	27	38	47	*	*	*	*	*	*	38	37
11:00	*	*	*	*	22	24	36	18	*	*	*	*		*	29	21
Total	0	0	0	0	1615	1568	1704	1655	0	0	0	0	0	0	1660	1613
Day	0	8 8	0		3183	3	3359	8 8	0	3 5	0		0		3273	
AM Peak					8:00	8:00	11:00	8:00							8:00	8:00
Volume					114	132	108	128							109	130
PM Peak		lie.			2:00	12:00 PM	1:00	2:00							1:00	2:00
ADIII IN P					0											

Site Code: 20260002

Location: Lamartine Street Location: West of Lodi Street City/State: Worcester, MA Direction: Combined

12:00 AM	rection: Com														
12:00 AM	7/18/2024														
1:00 0 0 0 4 12 12 12 4 2 0 0 0 0 0 0 0 0 0 0 2 2 2 0 0 0 0		MPH	20 MPH	25 MPH			40 MPH		50 MPH	55 MPH	60 MPH		70 MPH	MPH	Total
2200 0 0 0 1 6 4 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			1	3			1		0	0	0	0	0	0	3
3:00 0 0 2 10 4 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				4			4								34
4:00		0	0			4	1	0	0		0	0		0	12
6:00		0	0				3		0		0	0		0	19
6.00	4:00	0	0	2	17	12	6	2	0	0	0	0	0	0	39
7:00	5:00	2		4				3	1	0	0	0		0	70
8:00 11 29 33 86 56 15 2 0 0 0 0 0 0 0 2: 9:00 5 9 31 53 55 17 1 0 0 0 0 0 0 0 0 0 0 1 10:00 10 9 36 68 38 8 0 0 0 0 0 0 0 0 0 0 0 1 11:00 3 15 52 79 36 14 2 0 0 0 0 0 0 0 0 0 2 12:00 PM 4 14 36 81 61 20 3 0 0 0 0 0 0 0 0 0 2 1:00 16 26 64 81 51 5 0 0 0 0 0 0 0 0 0 0 0 2 2:00 22 33 69 49 20 5 0 0 0 0 0 0 0 0 0 0 0 1 3:00 0 4 23 48 89 32 8 0 0 0 0 0 0 0 0 0 0 1 3:00 0 0 4 23 48 89 32 8 0 0 0 0 0 0 0 0 0 0 1 4:00 6 5 33 56 94 42 7 6 0 0 0 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 0 2 6:00 0 2 10 38 57 36 8 2 2 0 0 0 0 0 0 0 0 1 7:00 0 0 10 55 68 28 2 2 2 0 0 0 0 0 0 0 1 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 0 1 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 0 1 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 1 10:00 1 0 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 12:00 0 0 0 0 0 0 0 0 0 13:00 0 0 0 0 0 0 0 0 0 14:00 0 0 0 0 0 0 0 15:00 0 0 0 0 0 0 0 0 16:00 0 0 0 0 0 0 0 17:00 0 0 0 0 0 0 0 0 18:00 0 0 0 0 0 0 0 0 19:00 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 0 0 0 0 10:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	6:00	2	6	10	39	49	18	4	1	0	0	0	0	0	129
9:00	7:00	19	22	59	56	20	2	0	0	0	0	0	0	0	178
10:00 10 9 36 68 38 8 0 0 0 0 0 0 0 0 0 0 11 11:00 3 15 52 79 36 14 2 0 0 0 0 0 0 0 0 0 0 2 12:00 PM 4 14 36 81 61 20 3 0 0 0 0 0 0 0 0 0 2 1:00 16 26 64 81 51 5 0 0 0 0 0 0 0 0 0 0 2 2:00 22 33 69 49 20 5 0 0 0 0 0 0 0 0 0 0 0 3:00 0 4 23 48 89 32 8 0 0 0 0 0 0 0 0 0 0 2 4:00 6 5 33 56 94 42 7 6 0 0 0 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 0 0 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 0 0 0 1:0 7:00 0 0 10 55 68 28 2 2 0 0 0 0 0 0 0 0 1 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 0 1 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 0 0 1 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 1 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 3 4 1 0 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 3 4 1 0 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 0 0 0 11:00 1 0 10 35 28 6 5 17 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 0 0 11:00 1 10 10 10 35 28 6 6 4 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8:00	11	29	33	86	56	15	2	0	0	0	0	0	0	232
11:00	9:00	5	9	31	53	55	17	1	0	0	0	0	0	0	171
12:00 PM	10:00	10	9	36	68	38	8	0	0	0	0	0	0	0	169
1:00	11:00	3	15	52	79	36	14	2	0	0	0	0	0	0	201
1:00	12:00 PM	4	14	36	81	61	20	3	0	0	0	0	0	0	219
3:00		16	26	64	81	51	5	0	0	0	0	0	0	0	243
4:00 6 5 33 56 94 42 7 6 0 0 0 0 0 0 2 2 5:00 1 9 18 59 90 40 10 1 0 0 0 0 0 0 0 2 2 6:00 0 0 2 10 38 57 36 8 2 0 0 0 0 0 0 0 0 13 7:00 0 0 0 10 55 68 28 2 2 2 0 0 0 0 0 0 0 0 11 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 0 0 13 9:00 0 6 14 57 45 16 2 2 0 0 0 0 0 0 0 11 9:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 11 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 0 11 10:00 1 0 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 1 11 10:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 0 1 11 10:00 1 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 3 33 10 10 10 10 10 10 10 10 10 10 10 10 10	2:00	22	33	69	49	20	5	0	0	0	0	0	0	0	198
5:00	3:00	0	4	23	48	89	32	8	0	0	0	0	0	0	204
6:00 0 2 10 38 57 36 8 2 0 0 0 0 0 0 18 7:00 0 0 10 55 68 28 2 2 0 0 0 0 0 0 18 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 18 9:00 0 6 14 57 45 16 2 2 0 0 0 0 0 0 0 11 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 1 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 50 0 5 17 0 0 0 0 0 0 0 0 0 11:00 0 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 0 12:00 0 0 0 0 0 0 0 0 0 0 0 0 13:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 14:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4:00	6	5	33	56	94	42	7	6	0	0	0	0	0	249
6:00 0 2 10 38 57 36 8 2 0 0 0 0 0 0 18 7:00 0 0 10 55 68 28 2 2 0 0 0 0 0 0 18 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 18 9:00 0 6 14 57 45 16 2 2 0 0 0 0 0 0 0 11 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 1 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 50 0 5 17 0 0 0 0 0 0 0 0 0 11:00 0 0 5 7 17 18 6 0 0 1 0 0 0 0 0 0 0 0 0 12:00 0 0 0 0 0 0 0 0 0 0 0 0 13:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 14:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5:00	1	9	18	59	90	40	10	1	0	0	0	0	0	228
7:00 0 0 10 55 68 28 2 2 0 0 0 0 0 0 16 8:00 4 13 13 39 46 16 3 0 0 0 0 0 0 0 0 13 9:00 0 6 14 57 45 16 2 2 0 0 0 0 0 0 13 10:00 1 0 10 35 28 6 4 1 0 0 0 0 0 0 0 1 11:00 0 5 7 17 18 6 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 1 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 1 0 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 6 0 1 0 0 0 0 0 0 0 0 0 11:00 0 5 7 17 18 50th 85th 95th  Speed 21 28 34 37  Mean Speed (Average) 28.9 10 MPH Pace Speed 26-35 Number in Pace 1968 Percent in Pace 61.0% Number > 30 MPH 1429 Percent > 30 MPH 42.5%  Stats Percentile 15th 50th 85th 95th  Speed 23 29 34 37  Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 63.0% Number > 30 MPH 3027		0	2	10			36		2	0	0	0			153
9:00	7:00	0	0	10	55	68	28	2		0	0	0	0	0	165
9:00	8:00	4	13	13	39	46	16	3	0	0	0	0	0	0	134
10:00															142
11:00 0 5 7 17 18 6 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0															85
Total 106 211 544 1069 997 350 65 17 0 0 0 0 0 0 338  Percentile 15th 50th 85th 95th Speed 21 28 34 37  Mean Speed (Average) 28.9 10 MPH Pace Speed 26-35 Number in Pace 1968 Percent in Pace 61.0% Number > 30 MPH 1429 Percent > 30 MPH 42.5%  Grand Total 163 331 959 2062 2086 760 153 26 2 0 0 0 0 0 654  Stats Percentile 15th 50th 85th 95th Speed 23 29 34 37  Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027									1						54
Percentile 15th 50th 85th 95th Speed 21 28 34 37  Mean Speed (Average) 28.9 10 MPH Pace Speed 26-35 Number in Pace 1968 Percent in Pace 61.0% Number > 30 MPH 42.5%  Grand Total 163 331 959 2062 2086 760 153 26 2 0 0 0 0 65- Stats Percentile 15th 50th 85th 95th Speed 23 29 34 37  Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027		106	211	544	1069		350	65	17	0		0	0	0	3359
Mean Speed (Average) 28.9 10 MPH Pace Speed 26-35 Number in Pace 1968 Percent in Pace 61.0% Number > 30 MPH 1429 Percent > 30 MPH 42.5%  Grand Total 163 331 959 2062 2086 760 153 26 2 0 0 0 0 0 654 Stats Percentile 15th 50th 85th 95th Speed 23 29 34 37  Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027			F	Percentile	15th	50th	85th	95th							
Mean Speed (Average) 28.9 10 MPH Pace Speed 26-35 Number in Pace 1968 Percent in Pace 61.0% Number > 30 MPH 1429 Percent > 30 MPH 42.5%  Grand Total 163 331 959 2062 2086 760 153 26 2 0 0 0 0 0 654 Stats Percentile 15th 50th 85th 95th Speed 23 29 34 37  Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027				Speed	21	28	34	37							
10 MPH Pace Speed 26-35 Number in Pace 1968 Percent in Pace 61.0% Number > 30 MPH 1429 Percent > 30 MPH 42.5%  Grand Total 163 331 959 2062 2086 760 153 26 2 0 0 0 0 0 65- Stats Percentile 15th 50th 85th 95th Speed 23 29 34 37  Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027		Mea	n Speed (	7-1000000000000000000000000000000000000	28.9										
Number in Pace					26-35										
Number > 30 MPH			Numbe	r in Pace	1968										
Percent > 30 MPH			Percer	t in Pace	61.0%										
Percent > 30 MPH			Number >	30 MPH	1429										
Grand Total         163         331         959         2062         2086         760         153         26         2         0         0         0         0         654           Stats         Percentile         15th         50th         85th         95th           Speed         23         29         34         37           Mean Speed (Average)         29.5         10 MPH Pace Speed         26-35           Number in Pace         4113         Percent in Pace         63.0%           Number > 30 MPH         3027			Percent >	30 MPH	42.5%										
Stats         Percentile         15th         50th         85th         95th           Speed         23         29         34         37           Mean Speed (Average)         29.5         10 MPH Pace Speed         26-35           Number in Pace         4113         Percent in Pace         63.0%           Number > 30 MPH         3027	Grand Total	163				2086	760	153	26	2	0	0	0	0	6542
Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027		11.00	F	Percentile	15th	50th	85th	95th					,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
Mean Speed (Average) 29.5 10 MPH Pace Speed 26-35 Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027				Speed	23	29	34	37							
10 MPH Pace Speed 26-35  Number in Pace 4113  Percent in Pace 63.0%  Number > 30 MPH 3027		Mea	n Speed (												
Number in Pace 4113 Percent in Pace 63.0% Number > 30 MPH 3027															
Percent in Pace 63.0% Number > 30 MPH 3027															
Number > 30 MPH 3027															
					46.3%										

Site Code: 20260003 Accurate Counts 978-664-2565

verage	WB,	32	25	13	12	18	36	81	126	173	148	152	146	158	174	170	152	181	163	150	154	106	112	75	40	2597	80	8:00	173	4:00	181
Week Average	EB,	12	တ	00	00	œ	20	58	63	2	99	77	99	72	84	79	108	109	102	70	64	47	54	28	19	1301	3898	10:00	77	4:00	109
Sunday	WB,	*	*	*	*	*	*	*	ĸ	*	ĸ	*	4	*	*	*	*	*	*	*	*	*	*	*	*	0					
Sur	EB,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0					
Saturday	WB,	*	*	*	*	*	*			*	*	*				*	*	*	*	*	*	*		*	*	0	0				
SS	EB,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	-				
Friday	EB, WB,	-14	*	*	*	*	*	*		*	٠	*	*	*	•	*	*	*	*	*	*	*	*	*	*	0	0				
	/B,	25	26	ıo	18	51	40	18	114	150	160	140	156	179	161	154	156	187	167	147	173	138	141	69	51	2659		00:6	160	4:00	187
Thursday	EB,	80	o	9	00	o	9	9	55	73	69	82	99	71	96	82	104	123	102	76	99	48	62	8	19	1345	4004	10:00	82	4:00	123
esday	WB,	38	24	21	7	16	31	81	137	196	137	165	137	138	188	187	149	175	159	152	136	73	83	81	29	2540	10	8:00	196	1:00	
Wednesday	EB,	17	0	10	0	œ	22	56	71	68	62	72	65	72	73	76	113	95	102	65	63	46	47	21	19	1261	3801	10:00	72	3:00	113
Tuesday	WB,	*	*	*	*	*	*	*	*	*	*	*	*	*		*	*	*	*	*	*	*	*	*	*	0 0	0				
Ĭ	EB,	*	*	*	*	*	*	*	*	*	*	*	+	*	*	*	*	*	*	*	*	*	*	*	*	0					
Monda	EB, WB,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0				
	Time	12:00 AM	1:00	2:00	3:00	4:00	5:00	00:9	7:00	8:00	00:6	10:00	11:00	12:00 PM	1:00	2:00	3:00	4:00	9:00	9:00	7:00	8:00	00:6	10:00	11:00	Total	Day	AM Peak	Volume	PM Peak	Volume

Location: Lamartine Street Site Code: 20260003

Location: Lamartine Street Location: East of Lodi Street City/State: Worcester, MA Direction: Combined

7/18/2024	0 - 3	> 3 - 6		> 9 - 12	> 12 - 15	> 15 - 18	> 18 - 21	> 21 - 24	> 24 - 27	> 27 - 30	> 30 - 33	> 33 - 36	> 36 - 39	> 39	
Time	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	Total
12:00 AM	0	0	0	0	3	8	14	5	2	0	1	0	0	0	33
1:00	0	0	1	1	6	6	9	7	2	2	1	0	0	0	35
2:00	0	0	1	0	4	0	1	2	2	0	0	0	0	1	11
3:00	0	0	0	1	2	4	8	8	2	1	0	0	0	0	26
4:00	0	0	0	0	0	4	11	9	5	1	0	0	0	0	30
5:00	0	0	0	2	2	5	21	10	10	7	1	0	0	0	58
6:00	0	0	0	8	5	15	36	30	30	12	3	1	0	0	140
7:00	0	0	0	3	22	27	49	30	24	11	3	0	0	0	169
8:00	0	0	3	10	18	25	78	38	33	14	4	0	0	0	223
9:00	0	0	4	4	26	39	68	47	21	16	4	0	0	0	229
10:00	0	0	1	12	32	55	65	23	16	11	6	0	0	1	222
11:00	0	0	0	9	22	51	65	42	22	7	4	0	0	0	222
12:00 PM	0	0	3	14	31	57	76	33	22	7	6	0	1	0	250
1:00	0	0	7	4	22	45	80	38	35	18	7	1	0	0	257
2:00	0	0	7	16	22	39	69	52	19	9	3	0	0	0	236
3:00	0	0	3	2	24	33	81	55	46	13	2	1	0	0	260
4:00	0	0	1	6	26	53	81	70	39	24	9	0	0	1	310
5:00	0	0	3	7	19	32	82	54	41	23	5	1	1	1	269
6:00	0	0	4	4	21	41	67	32	28	21	3	0	1	1	223
7:00	0	0	1	3	17	36	81	56	30	10	2	2	1	0	239
8:00	0	0	1	3	22	39	63	27	18	8	3	2	0	0	186
9:00	0	0	0	3	32	30	68	36	19	12	2	1	0	0	203
10:00	0	0	3	1	12	21	20	25	13	5	1	1	1	0	103
11:00	0	0	2	2	9	19	14	10	7	4	3	0	0	0	70
Total	0	0	45	115	399	684	1207	739	486	236	73	10	5	5	4004
		Р	ercentile	15th	50th	85th	95th								
		_	Speed	15	19	24	28								
		Speed (A		20.6											
	10 1	MPH Pac	AND REAL PROPERTY.	16-25											
			in Pace	2692											
			in Pace	70.0%											
		umber >		2761											
0 17.1		ercent >		69.0%	700	1010	2000	4404	070	100	400			40	7005
Grand Total	0	0	100	224	798	1312	2303	1484	976	433	133	23	7	12	7805
Stats		Р	ercentile	15th	50th	85th	95th								
	B. #100000.0000		Speed	15	19	24	27								
		Speed (A		20.6											
	101	MPH Pac	The state of the s	16-25											
			in Pace	5408											
	K1		in Pace	70.0%											
		umber >		5371											
		ercent >	18 MPH	68.8%											

Site Code: 20260004 Week Average EB, WB, 8:00 102 5:00 80 2471 Saturday WB, Accurate Counts 978-664-2565 Wednesday WB, 8:00 1:00 85 2393 0 AADT: 2,472 0 ADT: 2,472 7:00 8:00 10:00 11:00 11:00 10:0 Volume
PM Peak
Volume
Comb Total

Location: Green Street Site Code: 20260004

Location: Green Street Location: East of Lodi Street City/State: Worcester, MA Direction: Combined

7/18/2024	0 - 3	> 3 - 6	> 6 - 9	> 9 - 12	> 12 - 15	> 15 - 18	> 18 - 21	> 21 - 24	> 24 - 27	> 27 - 30	> 30 - 33	> 33 -	> 36 - 39	> 39	
Time	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	Total
12:00 AM	0	0	0	0	1	4	8	11	4	2	1	0	0	0	31
1:00	0	0	0	2	0	0	4	9	6	1	1	0	0	0	23
2:00	0	0	0	0	0	2	3	1	2	0	0	0	0	0	8
3:00	0	0	1	2	2	2	2	2	2	0	0	0	0	0	13
4:00	0	0	0	0	1	1	12	8	4	0	1	0	0	0	27
5:00	0	0	0	0	4	5	17	17	15	9	1	0	0	0	68
6:00	0	0	0	0	3	10	28	34	17	7	4	0	0	1	104
7:00	0	0	0	1	8	7	29	33	29	4	3	0	0	0	114
8:00	0	0	4	8	8	11	43	45	36	12	5	0	0	3	175
9:00	0	0	2	7	6	13	29	28	23	12	3	0	0	1	124
10:00	0	0	4	2	14	12	39	44	24	5	0	0	0	1	145
11:00	0	0	6	4	16	24	51	51	20	9	0	1	1	2	185
12:00 PM	0	0	0	4	13	20	41	44	25	9	2	1	0	1	160
1:00	0	0	5	7	15	34	49	36	26	6	1	0	0	0	179
2:00	0	0	1	3	6	25	47	41	29	8	1	1	0	0	162
3:00	0	0	2	3	11	11	30	34	27	5	2	1	0	0	126
4:00	0	0	0	1	14	33	48	38	35	11	3	0	0	0	183
5:00	0	0	0	1	12	23	38	40	38	15	4	1	1	1	174
6:00	0	0	1	0	9	8	40	28	30	17	3	1	0	0	137
7:00	0	0	2	0	5	12	19	28	25	9	2	0	0	0	102
8:00	0	0	0	0	6	10	24	25	21	8	4	0	0	0	98
9:00	0	0	0	0	4	8	32	33	10	5	0	0	0	0	92
10:00	0	0	0	1	8	11	26	19	8	6	0	0	0	0	79
11:00	0	0	0	3	3	5	16	9	3	2	0	0	0	0	41
Total	0	0	28	49	169	291	675	658	459	162	41	6	2	10	2550
		Р	ercentile	15th	50th	85th	95th								
		0 1//	Speed	16	20	24	28								
			Average)	21.7											
	10 1		e Speed	18-27											
			in Pace	1811 75.0%											
	N		in Pace												
			21 MPH 21 MPH	1338 52.5%											
Grand Total	0	ercent >	39	81	311	562	1349	1287	859	338	79	18	3	17	4943
Stats	U		ercentile	15th	50th	85th	95th	1201	000	336	13	10	3	17	4945
Olais			Speed	16	20	24	28								
	Mean	Speed (4	Average)	21.8	20	24	20								
			e Speed	18-27											
	151		in Pace	3676											
			in Pace	75.0%											
	N	umber >	21 MPH	2601											

Accurate Counts 978-664-2565

Site Code: 20260005	ge Mon - Sun	0	2	2	2	_	9	10	16	14	24	19	19	30	16	18	14	20	20	12	œ	12	9	က	4	278		00:6	24 12:00 BM	30
Site Coo	Average Mon - Fri Mo	0	2	2	2	~	9	10	16	14	24	19	19	30	16	18	14	20	20	12	œ	12	9	က	4	278		00:6	24 12:00 BM	30
	7/21/2024 Sun	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	%0.0			
	7/20/2024 Sat	*	*	*	*	*	•	*	*	*	*	*	*	*	*	*	*	*	+	*	*	*	*	*	•	0	%0.0			
	7/19/2024 Fri	*	*	*	*	*	•	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	4	0	%0.0			
	7/18/2024 Thu	0	_	2	_	•	4	12	14	6	21	16	19	33	16	23	13	33	83	ത	11	10	တ	4	5	289	51.7%	00:6	21 12-00 DM	33
	7/17/2024 Wed	_	4	2	2	<u>.</u>	7	œ	17	20	28	22	19	27	16	14	16	80	18	15	9	13	n	2	2	270	48.3%	00:6	12:00 DM	12.00 FM 27
Street	7/16/24 Tue	*	*	*	*	*		*	*	*	*	*	*	•к	*	*	*	*	*	*	*	*	*	*	*	0	%0.0			
Street of Lamartine Sester. MA	7/15/24 Mon	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	<b>*</b>	*	*	*	*	*	*	0	%0.0			
Location: Lodi Street Location: South of Lamartine Street City/State: Worcester: MA	7/15/2024 Time	12:00 AM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	00:6	10:00	11:00	12:00 PM	1:00	2:00	3:00	4:00	2:00	6:00	7:00	8:00	00:6	10:00	11:00	Total	Percent	AM Peak	Volume	Volume

## **Accurate Counts** 978-664-2565

Location: Lodi Street Location: South of Lamartine Street City/State: Worcester, MA Direction: SB,

irection: SB,															
7/18/2024					> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -		
	0 - 3	> 3 - 6	> 6 - 9		15	18	21	24	27	30	33	36	39	> 39	
Time	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0		0	0	C
1:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
2:00	0	0	0	0	2	0	0	0	0	0	0		0	0	2
3:00	0	0	0	0	0	1	0		0	0	0		0	0	1
4:00	0	0	0	0	0	0	1	0	0	0	0		0	0	1
5:00	0	0	1	0	2	0	1	0	0	0	0	0	0	0	4
6:00	0	0	3	3	0	4	0	2	0	0	0	0	0	0	12
7:00	0	0	4	4	1	2	1	1	1	0	0		0	0	14
8:00	0	0	3	2	1	2	1	0	0	0	0		0	0	9
9:00	0	0	2	4	6	6	2	0	1	0	0		0	0	21
10:00	0	0	2	4	4	3	3	0	0	0	0	0	0	0	16
11:00	0	0	2	5	8	1	3	0	0	0	0	0	0	0	19
12:00 PM	0	0	5	5	12	6	2	1	0	0	1	0	0	1	33
1:00	0	0	3	3	4	3	2	0	1	0	0		0	0	16
2:00	0	0	2	5	8	3	2	1	0	0	1	0	0	1	23
3:00	0	0	2	0	6	2	2	1	0	0	0	0	0	0	13
4:00	0	0	6	6	5	7	1	2	1	0	0	0	2	3	33
5:00	0	0	2	5	10	2	3	0	1	0	0	0	0	0	23
6:00	0	0	1	2	0	1	4	1	0	0	0	0	0	0	9
7:00	0	0	3	2	3	0	0	0	0	0	0	0	2	1	11
8:00	0	0	2	2	2	1	1	0	0	0	0	0	0	2	10
9:00	0	0	0	0	3	4	0	0	0	0	0	0	0	2	9
10:00	0	0	1	0	0	1	2	0	0	0	0	0	0	0	4
11:00	0	0	1	0	2	0	1	1	0	0	0		0	0	5
Total	0	0	45	52	80	49	32	10	5	0	2	0	4	10	289
		P	ercentile	15th	50th	85th	95th								
	BI BI IND AV 100		Speed	8	13	16	20								
		Speed (A		15.5											
	10 1	MPH Pac	12 16	8-17											
			in Pace	194											
	640		in Pace	69.0%											
		umber >		192											
		ercent >		66.4%											
Grand Total	0	0	90	123	131	99	61	17	7	3	4	0	5	19	559
Stats		Р	ercentile	15th	50th	85th	95th								
	MARK THE STORY	_	Speed	8	12	16	19								
		Speed (A		15.2											
	10 1	ИРН Рас		10-19											
		Number	in Pace	382											
		Percent	in Pace	69.0%											
			in Pace 12 MPH												

Site Code: 20260006 Week Average SB, NB, 00:7 Sunday NB, Saturday NB, Friday Accurate Counts 978-664-2565 ω π ∞ ο <u>1</u> 1 1 τ ∞ <u>1</u> 1 4 ∞ π τ τ <del>1</del> 2 0 π ο 11:00 13 7:00 20 Thursday NB, 10:00 5 7:00 12 248 8:00 12:00 14:00 14:00 Wednesday SB, NB, 2:00 11:00 AADT: 235 Tuesday NB Location : Grosvenor Street Between
Location : Lamartine Street and Lafayette Street
City/State: Worcester. MA
7/15/2024 Monday
Time SB, NB, SB, N
12:00 AM
1:00 A
10:00 B
8:00 B ADT: 235 Volume Comb Total ADT Volume PM Peak

## **Accurate Counts** 978-664-2565

Location: Grosvenor Street Between Location: Lamartine Street and Lafayette Street City/State: Worcester, MA Direction: Combined

ection: Com	bined														
7/18/2024	0 - 3	> 3 - 6	> 6 - 9	> 0 12	> 12 - 15	> 15 - 18	> 18 - 21	> 21 - 24	> 24 - 27	> 27 - 30	> 30 -	> 33 -	> 36 - 39	> 39	
Time	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00	0	0	0	0	0	0	2	0	0	0	0	0	0	0	2
3:00	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
4:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
5:00	0	0	0	0	0	0	1	2	0	0	0	0	0	0	3
6:00	0	0	0	1	1	1	2	2	0	1	0	0	0	0	8
7:00	0	0	0	2	2	3	0	2	0	0	0	0	0	0	9
8:00	0	0	0	0	0	1	3	2	1	1	0	0	0	0	. 8
9:00	0	0	1	4	5	4	0	0	0	0	0	0	0	0	14
10:00	0	0	0	2	2	1	2	0	0	0	0	0	0	0	7
11:00	0	0	4	1	3	1	4	2	2	0	0	0	0	0	17
12:00 PM	0	0	0	2	1	3	3	1	1	0	0	0	0	0	11
1:00	0	0	3	4	2	2	3	2	0	1	0	0	0	0	17
2:00	0	0	0	4	4	3	4	3	0	0	0	0	0	0	18
3:00	0	0	1	2	3	4	1	1	0	0	0	0	0	0	12
4:00	0	0	1	3	5	5	4	0	0	0	0	0	0	0	18
5:00	0	0	1	2	2	5	5	2	1	0	0	0	0	0	18
6:00	0	0	2	1	3	4	4	1	2	0	0	0	0	0	17
7:00	0	0	14	5	3	3	4	1	2	0	0	0	0	0	32
8:00	0	0	0	4	4	1	4	1	0	0	0	0	0	0	14
9:00	0	0	3	0	1	3	0	1	0	0	0	0	0	0	8
10:00	0	0	1	3	2	2	0	0	0	0	0	0	0	0	8
11:00 Total	0	0	33	41	44	48	46	23	10	3	0	0	0	0	248
Total	0		ercentile	15th	50th	85th	95th	20	10	- 0	- 0		- 0	0	240
		670	Speed	9	15	20	23								
	Mean	Speed (A		15.5	1,000	(TES)	4779								
		MPH Pac		13-22											
			in Pace	149											
		Percent	in Pace	63.0%											
	N	umber >	15 MPH	130											
	P	ercent >	15 MPH	52.4%											
Grand Total	0	0	57	77	84	94	95	43	17	3	0	0	0	0	470
Stats		P	ercentile	15th	50th	85th	95th								
			Speed	9	15	20	23								
		Speed (A		15.6											
	10 N	ИРН Рас		13-22											
			in Pace	305											
			t in Pace	65.0%											
		umber >		252											
	Р	ercent >	15 MPH	53.6%											

Site Code: 20260007 Week Average SB, NB, 3:00 Sunday Friday Accurate Counts 978-664-2565 12:00 PM 11:00 18 3:00 15 ~ 400000000000 00:2 Wednesday NB. 12:00 PM 1 **AADT: 129** 

52

#### Accurate Counts 978-664-2565

Location: Meade Street Between

Location: Lamartine Street and Lafayette Street

Percent in Pace

Number > 12 MPH

Percent > 12 MPH

73.0%

57.8%

City/State: Worcester, MA

Direction: Combined > 36 -> 12 -> 15 -> 18 -> 21 -> 24 -> 27 -> 30 -> 33 -7/18/2024 0 - 3 > 3 - 6 > 6 - 9 > 9 - 12 > 39 Time MPH MPH MPH MPH **MPH MPH** MPH **MPH** MPH MPH MPH **MPH MPH** MPH Total 12:00 AM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 12:00 PM 1:00 2:00 3:00 4:00 5:00 6:00 7:00 8:00 9:00 10:00 11:00 Total Percentile 15th 50th 85th 95th Speed Mean Speed (Average) 13.5 10 MPH Pace Speed 7-16 Number in Pace 72.0% Percent in Pace Number > 12 MPH Percent > 12 MPH 58.2% **Grand Total** Percentile 15th 50th 85th 95th Stats Speed Mean Speed (Average) 13.5 10 MPH Pace Speed 7-16 Number in Pace 

Accurate Counts 978-664-2565

	ocation : Meade Street and Grosvenor Street City/State: Worcester, MA
	7/17/2024 7/18/2024 Wed Thu
7	7
9	9
4	4
က	က
5	5
27	27
38	38
30	30
44	44
40	40
61	61
45	45
38	38
61	61
47	47
09	09
65	65
51	51
51	51
45	45
32	32
31	31
28	28
12	12
831	831
48.3%	48.3%
10:00	10:00
61	61
4:00	
65	4:00

## Accurate Counts 978-664-2565

Location: Lafayette Street Between Location: Meade Street and Grosvenor Street City/State: Worcester, MA Direction: EB,

Direction: EB,															
7/18/2024					> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -		
	0 - 3		> 6 - 9		15	18	21	24	27	30	33	36	39	> 39	
Time	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	Total
12:00 AM	0	0	2	2	1	1	1	0	2	1	3	0	0	0	13
1:00	0	0	2	1	0	0	0	1	3	3	0	0	0	0	10
2:00	0	0	0	0	1	1	3	1	0	0	1	0	0	0	7
3:00	0	0	0	0	0	1	0	0	3	1	0	0	0	0	5
4:00	0	0	0	1	2	0	0	1	1	1	0	0	0	0	6
5:00	0	0	1	1	2	0	5	9	2	3	1	0	0	0	24
6:00	0	0	3	1	2	1	6	5	5	4	1	0	0	0	28
7:00	0	0	1	5	3	1	5	8	5	2	1	1	0	0	32
8:00	0	0	2	1	3	5	5	6	8	4	2	1	0	0	37
9:00	0	0	2	1	4	3	10	13	5	3	2	0	0	0	43
10:00	0	0	2	0	4	11	20	12	2	1	2	0	0	0	54
11:00	0	0	2	2	5	6	17	3	7	1	0	0	0	0	43
12:00 PM	0	0	6	6	5	9	19	10	3	3	0	0	0	0	61
1:00	0	0	3	1	4	3	18	13	5	2	0	1	1	0	51
2:00	0	0	1	1	3	5	13	14	11	5	0	1	0	0	54
3:00	0	0	4	1	7	8	12	11	4	1	1	0	0	0	49
4:00	0	0	3	1	5	7	14	15	6	2	2	0	0	0	55
5:00	0	0	4	5	10	3	10	12	8	2	0	1	1	0	56
6:00	0	0	3	5	6	9	16	7	7	2	2	1	0	1	59
7:00	0	0	0	1	5	5	16	10	11	4	1	0	0	0	53
8:00	0	0	2	4	6	7	9	13	7	1	1	0	0	0	50
9:00	0	0	4	0	4	3	15	8	2	1	3	0	1	1	42
10:00	0	0	4	2	3	2	7	6	5	0	1	2	0	0	32
11:00	0	0	1 50	0	2	1	6	7	2	3	2	0	0	1	25
Total	0	0	52	42	87	92	227	185	114	50	26	8	3	3	889
		P	ercentile	15th	50th	85th	95th								
	NA	C	Speed	20.4	6	12	25								
		Speed (A	5-100 Later 100	17-26											
	101	MPH Pac	and the same of th												
			in Pace	528 63.0%											
	N	umber >	t in Pace	708											
				79.6%											
Grand Total	0	ercent >	15 MPH	124	229	220	397	314	188	75	35	9	1	3	1720
Stats	U		ercentile	15th	50th	85th	95th	314	100	/5	35	9	4	3	1720
Stats		F	Speed	8	13	20	24								
	Maan	Speed (A		19.2	13	20	24								
		MPH Pac		16-25											
	101		in Pace	994											
			in Pace	59.0%											
	N	umber >		1245											
				72.4%											
	F	ercent >	13 IVIPH	12.470											



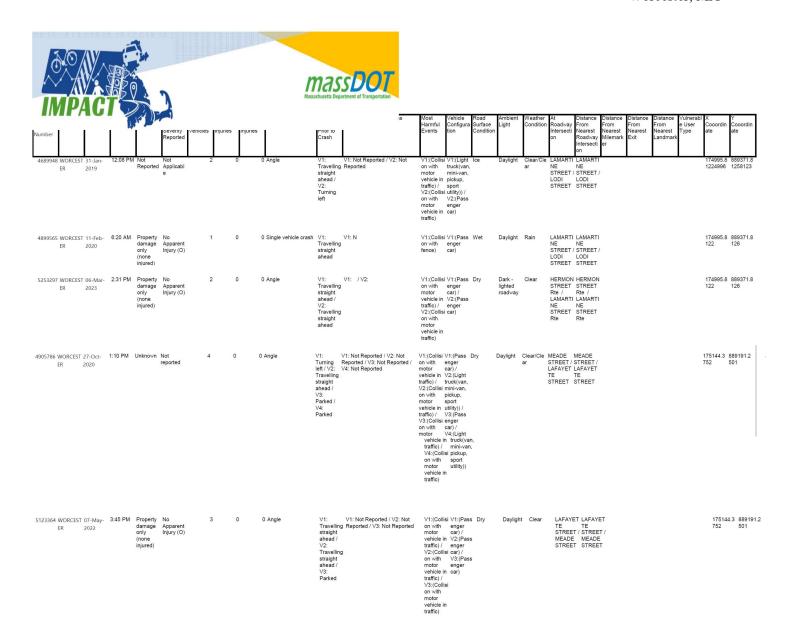
## INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN :		ALIZED :	X	of the control of the	TE :	Jul-24
MAJOR STREET :	Lafayette Str	eet				
MINOR STREET(S):	Meade Stree	t <mark>.</mark>				
( )						
INTERSECTION DIAGRAM	North		8	Meade St		
(Label Approaches)			Ĭ			
		Lafayette St	$\downarrow$			
		Lalayette St				-
		67	•			
			PEAK HOUF	VOLUMES		
APPROACH:	1	2	3	4	5	Total Peak Hourly
DIRECTION:	EB	WB	SB			Approach Volume
PEAK HOURLY VOLUMES (AM/PM):	67	0	8			75
"K" FACTOR:	0.090	INTERSI	ECTION ADT APPROACH		AL DAILY	833
TOTAL # OF CRASHES :	2	# OF YEARS :	5	CRASHES	GE#OF PERYEAR( ):	0.40
CRASH RATE CALCU	JLATION :	1.32	RATE =	(A*1,0	000,0 <mark>00 )</mark> * 365 )	
Comments : Much high						nassDOT
Project Title & Date:	39 Lamartine	Street reside	ntial developr	ment August	2024	



## INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN:	Worcester			COUNT DA	TE:	Jul-24
DISTRICT :	UNSIGN	ALIZED :	X	SIGNA	LIZED :	
		~ IN	TERSECTION	I DATA ~		
MAJOR STREET :	Green Island	Boulevard				
MINOR STREET(S):	Lamartine St	reet				
INTERSECTION DIAGRAM (Label Approaches)	North  Lamartine St	Hermon St	60 0 Lodi St	Green Island	d Blvd Lamartine St	
			PEAK HOUF	VOLUMES		
APPROACH:	1	2	3	4	5	Total Peak Hourly
DIRECTION:	NE	W	N	SW	S	Approach Volume
PEAK HOURLY VOLUMES (AM/PM) :	90	133	0	60	136	419
"K" FACTOR:	0.090	INTERSI	ECTION ADT APPROACH		AL DAILY	4,656
TOTAL # OF CRASHES :	3	# OF YEARS :	5	CRASHES	GE#OF PERYEAR( ):	0.60
CRASH RATE CALCU	JLATION :	0.35	RATE =	( A * 1,0	000,000 ) * 365 )	
Comments : Significant	y lower than the	e average rate	of 0.61 for unsi	gnalized inters	sections in Dist	3 of massDOt
Project Title & Date:	39 Lamartine	Street Resident	tial Developme	nt August 202	4	



Massachusetts Highway Department Statewide Traffic Data Collection 2019 Weekday Seasonal Factors

Factor Group	JAN	FEB	MAR	APR	MAY	NUL	JUL	AUG	SEP	ОСТ	NOV	DEC	Axle Factor
R1	1.22	1.14	1.12	1.06	1.00	96.0	0.87	0.85	96.0	0.99	1.04	1.12	98'0
R2	0.95	0.96	0.98	0.97	0.97	0.93	0.97	0.94	0.96	0.90	0.92	0.93	96.0
R3	1.15	1.06	1.07	1.00	0.89	0.88	0.89	0.89	0.95	0.92	1.02	1.01	76.0
R4-R7	1.09	1.09	1.11	1.02	0.96	0.92	0.89	0.89	0.99	0.98	1.09	1.13	86'0
U1-Boston	1.03	1.01	0.98	0.94	0.94	0.92	0.95	0.93	0.94	0.94	0.97	1.04	96'0
U1-Essex	1.09	1.06	1.03	0.99	0.94	06.0	0.88	0.86	0.93	0.94	0.99	1.06	66.0
U1-Southeast	1.06	1.05	1.01	76.0	0.95	0.93	0.93	06.0	0.94	0.94	0.98	1.04	0.98
U1-West	1.19	1.14	1.09	0.95	0.92	0.89	0.89	98.0	0.91	0.95	0.97	1.07	0.84
U1-Worcester	1.02	1.04	0.97	0.94	0.93	0.91	0.95	0.91	0.93	0.92	0.95	1.10	0.88
U2	1.01	1.00	0.94	0.93	0.91	0.89	0.93	06.0	06.0	0.91	0.94	1.02	66'0
U3	1.06	1.03	0.98	0.94	0.93	0.91	0.95	0.91	0.92	0.93	0.97	1.00	86.0
U4-U7	1.01	1.00	0.95	0.92	0.88	0.86	0.92	0.91	0.92	0.94	0.99	1.04	66.0
Rec - East	1.04	1.16	1.12	0.98	0.92	0.88	0.77	0.81	0.94	1.02	1.08	1.12	0.99
Rec - West	1.30	1.23	1.32	1.18	0.95	0.82	0.70	69.0	0.97	0.96	1.16	1.15	0.98

0-999 = 10Round off:

>1000 = 100

U = Urban

R = Rural

1 - Interstate

2 - Freeway and Expressway

3 - Other Principal Arterial

4 - Minor Arterial

5 - Major Collector

6 - Minor Collector

7 - Local Road and Street

7014,7079,7080,7090,7091,7092,7093,7094,7095,7096,7097,7108 and 7178), Martha's Vineyard and Nantucket. Recreational - East Group - Cape Cod (all towns) including the town of Plymouth south of Route 3A (stations

Recreational - West Group - Continuous Stations 2 and 189 including stations

1066, 1067, 1083, 1084, 1085, 1086, 1087, 1088, 1089, 1099, 1099, 1099, 1099, 1095, 1096, 1095, 1096, 1099, 1099, 1099, 1100, 1101, 1102, 1103, 1104, 1105, 1106, 1107, 1108, 1113, 1114, 1144, 1114, 1144, 1114, 1144, 1114, 1114, 1114, 1114, 1114, 1114, 1114, 1114, 1114, 1114, 1144, 1144, 1144, 1144, 1144, 1144, 1144, 1144, 1144, 1144, 1144

1116,2196,2197 and 2198.

## **MassDOT Yearly Growth Rates**

for data from 2014 to 2018

0 11					
Growth					
Factors					
Group	Grow 2014 to	Grow 2015 to	Grow 2016 to	Grow 2017 to	Grow 2018 to 2019
	2015	2016	2017	2018	
R1	0	0.023	0.004	0.018	0.016
R2	0.05	0.068	0.004	0.014	0.014
R3	-0.038	0.002	0.008	0.011	0.06
R4-7	-0.01	0.003	0.001	0.011	0.012
Rec – East		0.032	0.02	0.041	0.025
Rec – West		0.051	-0.008	0.029	0
U1-Boston	0.061	0.07	-0.003	0.012	0.006
U1-Essex	0.024	0.025	0.007	0.014	0.011
U1- Southeast	0.05	0.062	0.021	0.014	0
U1-West	0.03	-0.027	0.02	0.028	0.013
U1- Worcester	0.042	0.005	0.018	0.01	0.01
U2	0.04	0.048	0.008	0.01	0.02
U3	0.011	0.013	0.011	0.014	0.004
U4-7	0.023	0.062	0.017	0.003	-0.004

	Тур	Entering Vehicles, by Intersection be n queried on June 26, 2018)
Location	Signalized Intersections	Unsignalized Intersections
Statewide	0.78	0.57
District 1*	0.80*	0.44*
District 2	0.89	0.62
District 3	0.89	0.61
District 4	0.73	0.57
District 5	0.75	0.57
District 6	0.71	0.52
Location	Signa	lized Intersections

<sup>\* -</sup> District 1 should use Statewide Rates due to low sample total

Exhibit 3-8 Motor Vehicle Stopping Sight Distances

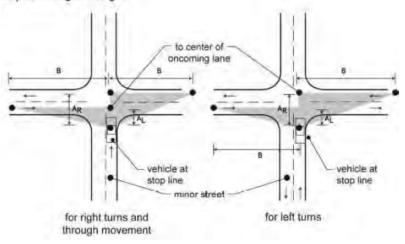
		Stop	ping Sight D	istance (ft) by	Percent Gra	de (%)	
	). <del>.</del>		Downgrade			Upgrade	
Design Speed	0	3	6	9	3	6	9
20	115	116	120	126	109	107	104
25	155	158	165	173	147	143	140
30	200	205	215	227	200	184	179
35	250	257	271	287	237	229	222
40	305	315	333	354	289	278	269
45	360	378	400	427	344	331	320
50	425	446	474	507	405	388	375
55	495	520	553	593	469	450	433
60	570	598	638	686	538	515	495
65	645	682	728	785	612	584	561
70	730	771	825	891	690	658	631
75	820	866	927	1003	772	736	704

Source: A Policy on Geometric Design of Streets and Highways, AASHTO, Washington DC, 2004. Chapter 3 Elements of Design



Exhibit 3-11 Sight Triangle Case B

## **Departure Sight Triangles**



Sight Triangle Legs: Case B - Stop Control on Cross Street

Length of Sight Triangle Legs (feet)

	1		0	
Major Street Design Speed (mph)	Minor Street for Vehicles Approaching From Right (AR, feet)	Minor Street for Vehicles Approaching From Left (AL, feet)	Major Street For Left Turns (B, feet)	Major Street for Right Turns or Through (B, feet)
15	32.5	20.5	170	145
20	32.5	20.5	225	195
25	32.5	20.5	280	240
30	32.5	20.5	335	290
35	32.5	20.5	390	335
40	32.5	20.5	445	385
45	32.5	20.5	500	430
50	32.5	20.5	555	480
55	32.5	20.5	610	530
60	32.5	20.5	665	575
65	32.5	20.5	720	625
70	32.5	20.5	775	670
75	32.5	20.5	830	720

Sight triangle legs shown are for passenger car crossing or turning into a two-lane street, with grades (all approaches) 3 percent or less. For other grades and for other major street widths, recalculate using AASHTO Green Book formulas.

Source: A Policy on Geometric Design of Streets and Highways, AASHTO, Washington DC, 2004. Chapter 3 Elements of Design

# Multifamily Housing - 2+ BR (Mid-Rise) Not Close to Rail Transit (221)

Peak Period Parking Demand vs: Dwelling Units

On a: Weekday (Monday - Friday)

Setting/Location: Dense Multi-Use Urban

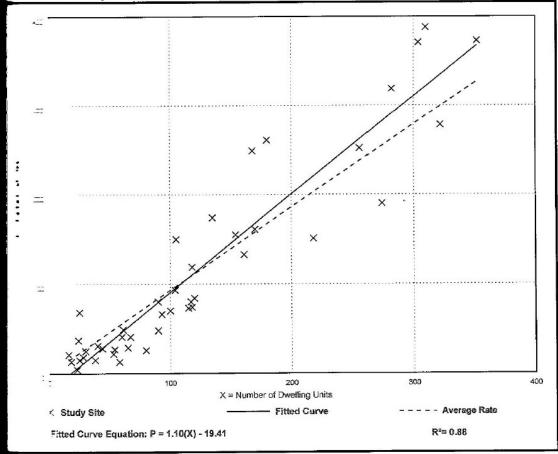
Number of Studies: 44

Avg. Num. of Dwelling Units: 120

## Period Parking Demand per Dwelling Unit

⊶erage Rate	Range of Rates	33rd / 85th Percentile	95% Confidence Interval	Standard Deviation (Coeff. of Variation)
1 93	0.17 - 2.72	0.68 / 1.29	0.83 - 1.03	0.33 ( 35% )





NC :-

# Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

Setting/Location: Dense Multi-Use Urban

Number of Studies: 3

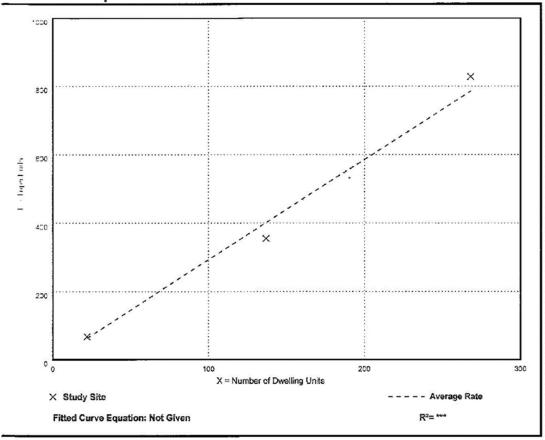
Avg. Num. of Dwelling Units: 142

Directional Distribution: 50% entering, 50% exiting

## Fehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
2.93	2.59 - 3.14	0.29

#### **Pata Plot and Equation**





# Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

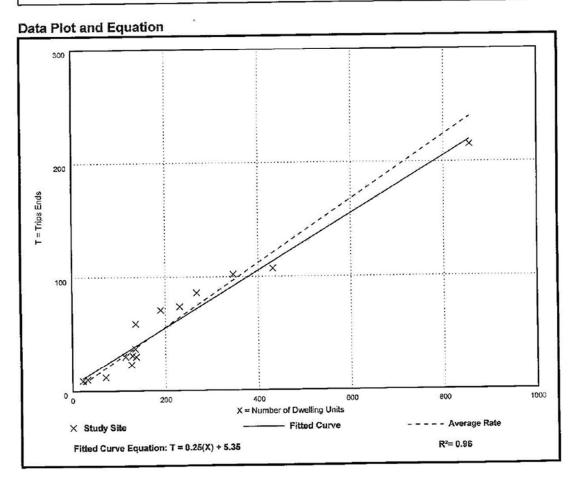
Setting/Location: Dense Multi-Use Urban

Number of Studies: 15 Avg. Num. of Dwelling Units: 215

Directional Distribution: 15% entering, 85% exiting

## Vehicle Trip Generation per Dwelling Unit

Action tith ocupration be		
Average Rate	Range of Rates	Standard Deviation
0.28	0.17 - 0.43	0.06





## Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: Dense Multi-Use Urban

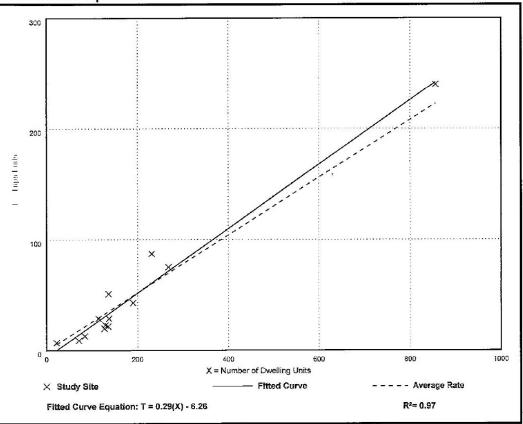
Number of Studies: 13 Avg. Num. of Dwelling Units: 192

Directional Distribution: 73% entering, 27% exiting

## 'ehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.26	0.13 - 0.38	0.07

## **Cata Plot and Equation**





Intersection of Lama	artine S	St & M	eade S	St AM p	oeak E	xisting Co	nditions		
	<b>→</b>	•	•	+	•	~			
Movement	EBT	EBR	WBL	WBT	NBL	NBR			
Lane Configurations	cî cî			र्स	N.				
Sign Control	Free			Free	Stop				
Grade	0%			0%	0%				
Volume (veh/h)	70	5	3	103	14	9			
Peak Hour Factor	0.88	0.63	0.36	0.89	0.54	0.50			
Hourly flow rate (veh/h)	80	8	8	116	26	18			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume			87		216	84			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)			4.1		6.4	6.2			
tC, 2 stage (s)									
tF (s)			2.2		3.5	3.3			
p0 queue free %			99		97	98			
cM capacity (veh/h)			1508		768	976			
Direction, Lane #	EB 1	WB 1	NB 1						
Volume Total	87	124	44						
Volume Left	0	8	26						
Volume Right	8	0	18						
cSH	1700	1508	841						
Volume to Capacity	0.05	0.01	0.05						
Queue Length (ft)	0	0	4						
Control Delay (s)	0.0	0.5	9.5						
Lane LOS		Α	Α						
Approach Delay (s)	0.0	0.5	9.5						
Approach LOS			А						
Intersection Summary									
Average Delay			1.9						
Intersection Capacity Uti	lization		17.1%	IC	CU Leve	el of Service		Α	

	-	*	1	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1			4	M		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	101	5	10	121	6	3	
Peak Hour Factor	0.77	0.63	0.75	0.94	0.34	0.42	
Hourly flow rate (veh/h)	131	8	13	129	18	7	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			139		291	135	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		97	99	
cM capacity (veh/h)			1444		694	914	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	139	142	25				
Volume Left	0	13	18				
Volume Right	8	0	7				
cSH	1700	1444	745				
Volume to Capacity	0.08	0.01	0.03				
Queue Length (ft)	0	1	3				
Control Delay (s)	0.0	0.8	10.0				
Lane LOS		Α	Α				
Approach Delay (s)	0.0	0.8	10.0				
Approach LOS			Α				
Intersection Summary							
Average Delay			1.2				
Intersection Capacity Uti	lization		18.7%	10	CILLEVE	of Service	Α

	$\rightarrow$	*	1	4	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>			4	W		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	70	0	5	113	6	4	
Peak Hour Factor	0.92	0.25	0.42	0.86	0.75	0.50	
Hourly flow rate (veh/h)	76	0	12	131	8	8	
Pedestrians	,,		-	101		0	
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)					NOTIC		
vC, conflicting volume			76		231	76	
vC1, stage 1 conf vol			10		201	10	
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
			4.1		0.4	0.2	
tC, 2 stage (s)			2.2		3.5	3.3	
tF (s)			99		99	99	
p0 queue free %			1523		751	985	
cM capacity (veh/h)					751	900	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	76	143	16				
Volume Left	0	12	8				
Volume Right	0	0	8				
cSH	1700	1523	852				
Volume to Capacity	0.04	0.01	0.02				
Queue Length (ft)	0	1	1				
Control Delay (s)	0.0	0.7	9.3				
Lane LOS		Α	Α				
Approach Delay (s)	0.0	0.7	9.3				
Approach LOS			Α				
Intersection Summary							
Average Delay			1.0				
Intersection Capacity Uti	lization		18.6%	10	CILLEVE	el of Service	

Intersection of Lamartine St & Grosvenor St PM peak Existing Conditions										
	-	•	•	<b>←</b>	•	-				
Movement	EBT	EBR	WBL	WBT	NBL	NBR				
Lane Configurations	1			લ	W					
Sign Control	Free			Free	Stop					
Grade	0%			0%	0%					
Volume (veh/h)	105	1	2	125	8	2				
Peak Hour Factor	0.78	0.25	0.50	0.85	0.63	0.50				
Hourly flow rate (veh/h)	135	4	4	147	13	4				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume			139		292	137				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)			4.1		6.4	6.2				
tC, 2 stage (s)										
tF (s)			2.2		3.5	3.3				
p0 queue free %			100		98	100				
cM capacity (veh/h)			1445		697	912				
Direction, Lane #	EB 1	WB 1	NB 1							
Volume Total	139	151	17							
Volume Left	0	4	13							
Volume Right	4	0	4							
cSH	1700	1445	739							
Volume to Capacity	0.08	0.00	0.02							
Queue Length (ft)	0	0	2							
Control Delay (s)	0.0	0.2	10.0							
Lane LOS		Α	А							
Approach Delay (s)	0.0	0.2	10.0							
Approach LOS			А							
Intersection Summary										
Average Delay			0.7							
Intersection Capacity Uti	lization		18.3%	10	CU Leve	l of Service		Α		

Movement EBL EBT WBT WBR SBL SBR  Lane Configurations Sign Control Free Free Stop Grade 0% 0% 0%  Volume (veh/h) 6 37 0 0 3 0  Peak Hour Factor 0.85 0.80 0.92 0.92 0.33 0.33  Hourly flow rate (veh/h) 7 46 0 0 9 0  Pedestrians  Lane Width (ft)  Walking Speed (ft/s)  Percent Blockage Right turn flare (veh)  Median type None
Movement         EBL         EBT         WBT         WBR         SBL         SBR           Lane Configurations         Image: Control of the control
Lane Configurations         Image: Free processing transport of the control of
Sign Control         Free         Free         Stop           Grade         0%         0%         0%           Volume (veh/h)         6         37         0         0         3         0           Peak Hour Factor         0.85         0.80         0.92         0.92         0.33         0.33           Hourly flow rate (veh/h)         7         46         0         0         9         0           Pedestrians         Lane Width (ft)         Walking Speed (ft/s)         Percent Blockage           Right turn flare (veh)         Right turn flare (veh)         Free Stop         Stop         Free Stop
Grade         0%         0%           Volume (veh/h)         6         37         0         0         3         0           Peak Hour Factor         0.85         0.80         0.92         0.92         0.33         0.33           Hourly flow rate (veh/h)         7         46         0         9         0           Pedestrians         Lane Width (ft)           Walking Speed (ft/s)           Percent Blockage           Right turn flare (veh)
Volume (veh/h) 6 37 0 0 3 0  Peak Hour Factor 0.85 0.80 0.92 0.92 0.33 0.33  Hourly flow rate (veh/h) 7 46 0 0 9 0  Pedestrians  Lane Width (ft)  Walking Speed (ft/s)  Percent Blockage  Right turn flare (veh)
Peak Hour Factor       0.85       0.80       0.92       0.92       0.33       0.33         Hourly flow rate (veh/h)       7       46       0       0       9       0         Pedestrians       Lane Width (ft)         Walking Speed (ft/s)         Percent Blockage         Right turn flare (veh)
Hourly flow rate (veh/h) 7 46 0 0 9 0 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh)
Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh)
Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh)
Walking Speed (ft/s) Percent Blockage Right turn flare (veh)
Percent Blockage Right turn flare (veh)
Right turn flare (veh)
Median type None
Median storage veh)
vC, conflicting volume 0 60 0
vC1, stage 1 conf vol
vC2, stage 2 conf vol
tC, single (s) 4.1 6.4 6.2
tC, 2 stage (s)
tF (s) 2.2 3.5 3.3
p0 queue free % 100 99 100
cM capacity (veh/h) 1623 942 1085
Direction, Lane # EB 1 WB 1 SB 1
Volume Total 53 0 9
Volume Left 7 0 9
Volume Right 0 0 0
cSH 1623 1700 942
Volume to Capacity 0.00 0.00 0.01
Queue Length (ft) 0 0 1
Control Delay (s) 1.0 0.0 8.9
Lane LOS A A
Approach Delay (s) 1.0 0.0 8.9
Approach LOS A
Intersection Summary
Average Delay 2.1
Intersection Capacity Utilization 13.3% ICU Level of Service A

Intersection of Lafayette St & Meade St PM Peak Existing Conditions									
	٠	-	•	*	-	4			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations		ર્ન	1		M				
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Volume (veh/h)	3	64	0	0	8	0			
Peak Hour Factor	0.75	0.76	0.92	0.92	0.50	0.33			
Hourly flow rate (veh/h)	4	84	0	0	16	0			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume	0				92	0			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)	4.1				6.4	6.2			
tC, 2 stage (s)									
tF (s)	2.2				3.5	3.3			
p0 queue free %	100				98	100			
cM capacity (veh/h)	1623				906	1085			
Direction, Lane #	EB 1	WB 1	SB 1						
Volume Total	88	0	16						
Volume Left	4	0	16						
Volume Right	0	0	0						
cSH	1623	1700	906						
Volume to Capacity	0.00	0.00	0.02						
Queue Length (ft)	0	0	1						
Control Delay (s)	0.3	0.0	9.0						
Lane LOS	Α		Α						
Approach Delay (s)	0.3	0.0	9.0						
Approach LOS			Α						
Intersection Summary									
Average Delay			1.7						
Intersection Capacity Uti	lization		14.7%	IC	CU Leve	el of Service		Α	

Intersection of Lafayette St & Grosvenor St AM Peak Existing Conditions									
	۶	<b>→</b>	<b>←</b>	•	-	1			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations		ન	1		W				
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Volume (veh/h)	9	39	0	0	5	0			
Peak Hour Factor	0.75	0.75	0.92	0.92	0.62	0.33			
Hourly flow rate (veh/h)	12	52	0	0	8	0			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume	0				76	0			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol						1/2-1/2-20V			
tC, single (s)	4.1				6.4	6.2			
tC, 2 stage (s)									
tF (s)	2.2				3.5	3.3			
p0 queue free %	99				99	100			
cM capacity (veh/h)	1623				920	1085			
Direction, Lane #	EB 1	WB 1	SB 1						
Volume Total	64	0	8						
Volume Left	12	0	8						
Volume Right	0	0	0						
cSH	1623	1700	920						
Volume to Capacity	0.01	0.00	0.01						
Queue Length (ft)	1	0	1						
Control Delay (s)	1.4	0.0	8.9						
Lane LOS	Α		Α						
Approach Delay (s)	1.4	0.0	8.9						
Approach LOS			А						
Intersection Summary									
Average Delay			2.2						
Intersection Capacity Uti	ilization		13.4%	IC	CU Leve	el of Service		Α	

Intersection of Lafayette St & Grosvenor St PM Peak Existing Conditions										
	۶	<b>→</b>	-	•	-	4				
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		ર્સ	1		W					
Sign Control		Free	Free		Stop					
Grade		0%	0%		0%					
Volume (veh/h)	8	61	0	0	4	0				
Peak Hour Factor	0.50	0.80	0.92	0.92	0.50	0.33				
Hourly flow rate (veh/h)	16	76	0	0	8	0				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume	0				108	0				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)	4.1				6.4	6.2				
tC, 2 stage (s)										
tF (s)	2.2				3.5	3.3				
p0 queue free %	99				99	100				
cM capacity (veh/h)	1623				880	1085				
Direction, Lane #	EB 1	WB 1	SB 1							
Volume Total	92	0	8							
Volume Left	16	0	8							
Volume Right	0	0	0							
cSH	1623	1700	880							
Volume to Capacity	0.01	0.00	0.01							
Queue Length (ft)	1	0	1							
Control Delay (s)	1.3	0.0	9.1							
Lane LOS	Α		Α							
Approach Delay (s)	1.3	0.0	9.1							
Approach LOS			Α							
Intersection Summary										
Average Delay			1.9							
Intersection Capacity Uti	ilization		14.9%	IC	CU Leve	el of Service		Α		

Intersection of Lama	Intersection of Lamartine St, Hermon St, Lodi St & Green St AM Peak Existing Conditions											
	٠	<b>→</b>	•	•	•	•	4	<b>†</b>	-	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	27	80	39	12	58	8	6	55	58	6	33	8
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.85	0.85	0.85	0.82	0.82	0.82
Hourly flow rate (veh/h)	31	92	45	13	62	9	7	65	68	7	40	10
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	168	84	140	57								
Volume Left (vph)	31	13	7	7								
Volume Right (vph)	45	9	68	10								
Hadj (s)	-0.1	0.0	-0.2	0.0								
Departure Headway (s)	4.3	4.6	4.3	4.6								
Degree Utilization, x	0.20	0.11	0.17	0.07								
Capacity (veh/h)	797	575	798	747								
Control Delay (s)	8.4	8.1	8.1	7.9								
Approach Delay (s)	8.4	8.1	8.1	7.9								
Approach LOS	Α	А	А	Α								
Intersection Summary												
Delay			8.2									
HCM Level of Service			Α									
Intersection Capacity Uti	lization		30.9%	10	CU Leve	el of Ser	vice		Α			

Intersection of Lamartine St, Ggreen Island Blvd, Lodi and Hermon Sts PM Peak Existing Conditions

	٠	<b>→</b>	•	1	+	•	1	<b>†</b>	~	1	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	7	52	31	22	34	4	14	49	70	16	103	17
Peak Hour Factor	0.89	0.89	0.89	0.79	0.79	0.79	0.81	0.81	0.81	0.76	0.76	0.76
Hourly flow rate (veh/h)	8	58	35	28	43	5	17	60	86	21	136	22
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	101	76	164	179								
Volume Left (vph)	8	28	17	21								
Volume Right (vph)	35	5	86	22								
Hadj (s)	-0.2	0.1	-0.3	0.0								
Departure Headway (s)	4.6	4.6	4.2	4.5								
Degree Utilization, x	0.13	0.10	0.19	0.22								
Capacity (veh/h)	730	574	810	778								
Control Delay (s)	8.2	8.1	8.2	8.7								
Approach Delay (s)	8.2	8.1	8.2	8.7								
Approach LOS	Α	А	Α	Α								
Intersection Summary												
Delay			8.4									
HCM Level of Service			Α									
Intersection Capacity Uti	lization		25.7%	Į.	CU Leve	el of Ser	vice		Α			

Intersection of Lamartine St & Meade St AM peak Future No Build Conditions										
	<b>→</b>	•	1	←	1	*				
Movement	EBT	EBR	WBL	WBT	NBL	NBR				
Lane Configurations	1			4	¥					
Sign Control	Free			Free	Stop					
Grade	0%			0%	0%					
Volume (veh/h)	77	6	4	113	15	10				
Peak Hour Factor	0.88	0.63	0.36	0.89	0.54	0.50				
Hourly flow rate (veh/h)	88	10	11	127	28	20				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume			97		241	92				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)			4.1		6.4	6.2				
tC, 2 stage (s)			0.0		2.5	2.2				
tF (s)			2.2		3.5	3.3				
p0 queue free %			99		96	98				
cM capacity (veh/h)			1496		741	965				
Direction, Lane #	EB 1	WB 1	NB 1							
Volume Total	97	138	48							
Volume Left	0	11	28							
Volume Right	10	0	20							
cSH	1700	1496	821							
Volume to Capacity	0.06	0.01	0.06							
Queue Length (ft)	0	1	5							
Control Delay (s)	0.0	0.7	9.7							
Lane LOS		Α	Α							
Approach Delay (s)	0.0	0.7	9.7							
Approach LOS			Α							
Intersection Summary										
Average Delay			1.9							
Intersection Capacity Uti	lization		18.2%	I	CU Leve	el of Service		A		

Intersection of Lamartine St & Meade St PM peak Future No Build Conditions										
	<b>→</b>	*	•	<b>←</b>	4	-				
Movement	EBT	EBR	WBL	WBT	NBL	NBR				
Lane Configurations	1>			4	¥					
Sign Control	Free			Free	Stop					
Grade	0%			0%	0%					
Volume (veh/h)	111	6	12	138	8	3				
Peak Hour Factor	0.77	0.63	0.75	0.94	0.34	0.42				
Hourly flow rate (veh/h)	144	10	16	147	24	7				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume			154		328	149				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)			4.1		6.4	6.2				
tC, 2 stage (s)										
tF (s)			2.2		3.5	3.3				
p0 queue free %			99		96	99				
cM capacity (veh/h)			1427		659	898				
Direction, Lane #	EB 1	WB 1	NB 1							
Volume Total	154	163	31							
Volume Left	0	16	24							
Volume Right	10	0	7							
cSH	1700	1427	703							
Volume to Capacity	0.09	0.01	0.04							
Queue Length (ft)	0	1	3							
Control Delay (s)	0.0	0.8	10.4							
Lane LOS		Α	В							
Approach Delay (s)	0.0	0.8	10.4							
Approach LOS			В							
Intersection Summary										
Average Delay			1.3							
Intersection Capacity Uti	ilization		20.5%	I	CU Leve	el of Service		A		

Intersection of Lama	Intersection of Lamartine St & Grosvenor St AM peak Future No Build Conditions										
	-	*	•	<b>←</b>	1	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR					
Lane Configurations	1>			4	N.						
Sign Control	Free			Free	Stop						
Grade	0%			0%	0%						
Volume (veh/h)	77	1	6	124	8	5					
Peak Hour Factor	0.92	0.25	0.42	0.86	0.75	0.50					
Hourly flow rate (veh/h)	84	4	14	144	11	10					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type					None						
Median storage veh)											
vC, conflicting volume			88		258	86					
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
tC, single (s)			4.1		6.4	6.2					
tC, 2 stage (s)											
tF (s)			2.2		3.5	3.3					
p0 queue free %			99		99	99					
cM capacity (veh/h)			1508		723	973					
Direction, Lane #	EB 1	WB 1	NB 1								
Volume Total	88	158	21								
Volume Left	0	14	11								
Volume Right	4	0	10								
cSH	1700	1508	826								
Volume to Capacity	0.05	0.01	0.03								
Queue Length (ft)	0	1	2								
Control Delay (s)	0.0	0.7	9.5								
Lane LOS		Α	Α								
Approach Delay (s)	0.0	0.7	9.5								
Approach LOS			Α								
Intersection Summary											
Average Delay			1.2								
Intersection Capacity Uti	lization		20.0%	10	CU Leve	l of Service	A				

Intersection of Lamartine St & Grosvenor St PM peak Future No Build Conditions									
	-	•	•	<b>←</b>	1	-			
Movement	EBT	EBR	WBL	WBT	NBL	NBR			
Lane Configurations	1			4	M				
Sign Control	Free			Free	Stop				
Grade	0%			0%	0%				
Volume (veh/h)	116	3	2	138	11	3			
Peak Hour Factor	0.78	0.25	0.50	0.85	0.63	0.50			
Hourly flow rate (veh/h)	149	12	4	162	17	6			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume			161		325	155			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)			4.1		6.4	6.2			
tC, 2 stage (s)									
tF (s)			2.2		3.5	3.3			
p0 queue free %			100		97	99			
cM capacity (veh/h)			1418		667	891			
Direction, Lane #	EB 1	WB 1	NB 1						
Volume Total	161	166	23						
Volume Left	0	4	17						
Volume Right	12	0	6						
cSH	1700	1418	713						
Volume to Capacity	0.09	0.00	0.03						
Queue Length (ft)	0	0	3						
Control Delay (s)	0.0	0.2	10.2						
Lane LOS		Α	В						
Approach Delay (s)	0.0	0.2	10.2						
Approach LOS			В						
Intersection Summary									
Average Delay			0.8						
Intersection Capacity Uti	lization		19.2%	I	CU Leve	of Service	A		

Intersection of Lafayette St & Meade St AM Peak Future No Build Conditions										
	۶	<b>→</b>	<b>←</b>	•	-	4				
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		4	1>		¥					
Sign Control		Free	Free		Stop					
Grade		0%	0%		0%					
Volume (veh/h)	8	41	0	0	3	0				
Peak Hour Factor	0.85	0.80	0.92	0.92	0.33	0.33				
Hourly flow rate (veh/h)	9	51	0	0	9	0				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume	0				70	0				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)	4.1				6.4	6.2				
tC, 2 stage (s)										
tF(s)	2.2				3.5	3.3				
p0 queue free %	99				99	100				
cM capacity (veh/h)	1623				929	1085				
Direction, Lane #	EB 1	WB 1	SB 1							
Volume Total	61	0	9							
Volume Left	9	0	9							
Volume Right	0	0	0							
cSH	1623	1700	929							
Volume to Capacity	0.01	0.00	0.01							
Queue Length (ft)	0	0	1							
Control Delay (s)	1.2	0.0	8.9							
Lane LOS	Α		Α							
Approach Delay (s)	1.2	0.0	8.9							
Approach LOS			Α							
Intersection Summary										
Average Delay			2.2							
Intersection Capacity Uti	ilization		13.3%	IC	CU Leve	el of Service		Α		

Intersection of Lafay	Intersection of Lafayette St & Meade St PM Peak Future No Build Conditions									
	•	<b>→</b>	•	*	-	4				
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		र्स	1>		N/F					
Sign Control		Free	Free		Stop					
Grade		0%	0%		0%					
Volume (veh/h)	4	70	0	0	10	0				
Peak Hour Factor	0.75	0.76	0.92	0.92	0.50	0.33				
Hourly flow rate (veh/h)	5	92	0	0	20	0				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume	0				103	0				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)	4.1				6.4	6.2				
tC, 2 stage (s)										
tF (s)	2.2				3.5	3.3				
p0 queue free %	100				98	100				
cM capacity (veh/h)	1623				892	1085				
Direction, Lane #	EB 1	WB 1	SB 1							
Volume Total	97	0	20							
Volume Left	5	0	20							
Volume Right	0	0	0							
cSH	1623	1700	892							
Volume to Capacity	0.00	0.00	0.02							
Queue Length (ft)	0	0	2							
Control Delay (s)	0.4	0.0	9.1							
Lane LOS	Α		Α							
Approach Delay (s)	0.4	0.0	9.1							
Approach LOS			Α							
Intersection Summary										
Average Delay			1.9							
Intersection Capacity Uti	lization		15.1%	10	CU Leve	el of Service		Α		

Intersection of Lafay	ette S	t & Gr	osven	or St A	M Pea	ık Future	No Build	Condition	ons
	۶	<b>→</b>	+	•	-	1			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
ane Configurations		ર્ન	f)		M				
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
olume (veh/h)	11	43	0	0	6	0			
eak Hour Factor	0.75	0.75	0.92	0.92	0.62	0.33			
lourly flow rate (veh/h)	15	57	0	0	10	0			
edestrians									
ane Width (ft)									
alking Speed (ft/s)									
ercent Blockage									
ght turn flare (veh)									
edian type					None				
edian storage veh)									
, conflicting volume	0				87	0			
1, stage 1 conf vol									
2, stage 2 conf vol									
single (s)	4.1				6.4	6.2			
, 2 stage (s)									
(s)	2.2				3.5	3.3			
queue free %	99				99	100			
1 capacity (veh/h)	1623				906	1085			
rection, Lane #	EB 1	WB 1	SB 1						
lume Total	72	0	10						
lume Left	15	0	10						
lume Right	0	0	0						
SH .	1623	1700	906						

0.01

1.5

1.5

Α

0.01

9.0

9.0 Α

2.4

13.8%

Α

0.00 0

0.0

0.0

Volume to Capacity
Queue Length (ft)
Control Delay (s)
Lane LOS

Approach Delay (s)
Approach LOS

Intersection Summary

Average Delay
Intersection Capacity Utilization

ICU Level of Service	A

Intersection of Lafayette St & Grosvenor St PM Peak Future No Build Conditions										
	٠	<b>→</b>	<b>←</b>	•	-	✓				
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		स्	1>		N/					
Sign Control		Free	Free		Stop					
Grade		0%	0%		0%					
Volume (veh/h)	10	67	0	0	6	0				
Peak Hour Factor	0.50	0.80	0.92	0.92	0.50	0.33				
Hourly flow rate (veh/h)	20	84	0	0	12	0				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume	0				124	0				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)	4.1				6.4	6.2				
tC, 2 stage (s)										
tF (s)	2.2				3.5	3.3				
p0 queue free %	99				99	100				
cM capacity (veh/h)	1623				861	1085				
Direction, Lane #	EB 1	WB 1	SB 1							
Volume Total	104	0	12							
Volume Left	20	0	12							
Volume Right	0	0	0							
cSH	1623	1700	861							
Volume to Capacity	0.01	0.00	0.01							
Queue Length (ft)	1	0	1							
Control Delay (s)	1.5	0.0	9.2							
Lane LOS	Α		Α							
Approach Delay (s)	1.5	0.0	9.2							
Approach LOS			Α							
Intersection Summary										
Average Delay			2.3							
Intersection Capacity Uti	ilization		15.5%	IC	CU Leve	el of Service	А			

Intersection of Lamartine St, Hermon St, Lodi St & Green St AM Peak Future No Build Condition

	•	<b>→</b>	•	1	<b>←</b>		1	<b>†</b>	-	-	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	30	88	43	15	64	9	7	61	66	7	37	9
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.85	0.85	0.85	0.82	0.82	0.82
Hourly flow rate (veh/h)	34	101	49	16	69	10	8	72	78	9	45	11
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	185	95	158	65								
Volume Left (vph)	34	16	8	9								
Volume Right (vph)	49	10	78	11								
Hadj (s)	-0.1	0.0	-0.3	0.0								
Departure Headway (s)	4.4	4.7	4.3	4.7								
Degree Utilization, x	0.23	0.12	0.19	0.08								
Capacity (veh/h)	771	567	781	729								
Control Delay (s)	8.7	8.3	8.4	8.1								
Approach Delay (s)	8.7	8.3	8.4	8.1								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			8.4									
HCM Level of Service			Α									
Intersection Capacity Uti	lization		33.5%	10	CU Leve	el of Ser	vice		Α			

## Intersection of Lamartine St, Ggreen Island Blvd, Lodi and Hermon Sts PM Peak Future No Build Conditions

	٠	<b>→</b>	*	1	<b>—</b>	•	1	<b>†</b>	~	1	Ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	8	57	35	25	37	4	15	55	80	18	114	19
Peak Hour Factor	0.89	0.89	0.89	0.79	0.79	0.79	0.81	0.81	0.81	0.76	0.76	0.76
Hourly flow rate (veh/h)	9	64	39	32	47	5	19	68	99	24	150	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	112	84	185	199								
Volume Left (vph)	9	32	19	24								
Volume Right (vph)	39	5	99	25								
Hadj (s)	-0.2	0.1	-0.3	0.0								
Departure Headway (s)	4.7	4.7	4.3	4.5								
Degree Utilization, x	0.15	0.11	0.22	0.25								
Capacity (veh/h)	711	566	794	763								
Control Delay (s)	8.5	8.3	8.5	9.0								
Approach Delay (s)	8.5	8.3	8.5	9.0								
Approach LOS	Α	Α	Α	Α								
Intersection Summary												
Delay			8.7									
HCM Level of Service			Α									
Intersection Capacity Uti	lization		29.0%	- 1	CU Leve	el of Ser	vice		Α			

## Intersection of Lamartine St & Meade St AM peak Future Build Conditions 1 EBR Movement **EBT** WBL **WBT** NBL **NBR** Lane Configurations Y 4 1 Sign Control Free Free Stop Grade 0% 0% 0% Volume (veh/h) 77 6 113 15 11 Peak Hour Factor 0.88 0.36 0.50 0.63 0.89 0.54 Hourly flow rate (veh/h) 88 11 127 28 22 10 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) None Median type Median storage veh) 97 241 92 vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol tC, single (s) 4.1 6.4 6.2 tC, 2 stage (s) tF(s) 2.2 3.5 3.3 p0 queue free % 99 96 98 cM capacity (veh/h) 1496 741 965 Direction, Lane # WB 1 NB 1 EB 1 Volume Total 97 138 50 Volume Left 0 11 28 Volume Right 10 22 0 cSH 1700 1496 826 Volume to Capacity 0.06 0.01 0.06 Queue Length (ft) 0 5 Control Delay (s) 0.0 0.7 9.6 Lane LOS Α Approach Delay (s) 0.0 0.7 9.6 Approach LOS A Intersection Summary

ICU Level of Service

Α

2.0

18.2%

Average Delay

Intersection Capacity Utilization

Movement	Intersection of Lama	rtine S	St & M	eade S	St PM	oeak F	uture Build	d Conditions		
Lane Configurations		_	>	-	<b>←</b>	•	<b>/</b>			
Lane Configurations	Movement	ERT	ERP	\V/BI	\M/RT	NRI	NIRP			
Sign Control         Free Grade         Free Own			LDIN	WDL	2.2 50.2		NDIX			
Grade 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0% 0%										
Volume (veh/h)         111         6         12         138         8         3           Peak Hour Factor         0.77         0.63         0.75         0.94         0.34         0.42           Hourly flow rate (veh/h)         144         10         16         147         24         7           Pedestrians         Lane Width (ft)         Walking Speed (ft/s)         Percent Blockage         Percent Blockage           Right turn flare (veh)         Median type         None         None           Median storage veh)         VC. conflicting volume         154         328         149           VC2, stage 2 conf vol         VC3, stage 2 conf vol         VC4, stage 2 conf vol         VC2, stage 2 conf vol         VC3, stage 2 conf vol         VC4, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC3, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC3, stage 2 conf vol         VC4, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC3, stage 2 conf vol         VC4, stage 2 conf vol         VC2, stage 2 conf vol         VC4, stage 2 conf vol										
Peak Hour Factor 0.77 0.63 0.75 0.94 0.34 0.42 Hourly flow rate (veh/h) 144 10 16 147 24 7 Pedestrians  Lane Width (ft)  Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type None Median storage veh) VC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol tC, single (s) tC, 2 stage (s) tF (s)			6	10			2			
Hourly flow rate (veh/h) 144 10 16 147 24 7 Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type Median storage veh) VC, conflicting volume vC1, stage 1 conf vol VC2, stage 2 conf vol tC, single (s) tC, 2 stage (s) tF (s) p0 queue free % p9 96 99 CM capacity (veh/h) 1427 659 898  Direction, Lane # EB 1 WB 1 NB 1 Volume Total Volume Left Volume Right 10 0 7 CSH 1700 1427 703 Volume to Capacity Volume to Capacity 0, 09 0.01 0.04 Queue Length (ft) 0 1 3 Control Delay (s) Lane LOS A B Approach Dolay (s) Intersection Summary Average Delay  1.3										
Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type										
Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median storage veh) VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol tC, single (s) tC, 2 stage (s) tF (s)		144	10	10	147	24	ı			
Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type None Median storage veh) VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC, 2 stage (s) tf. (s)										
Percent Blockage Right turn flare (veh) Median type  None Median storage veh) VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol tC, single (s)  tF (s)  2.2  3.5  3.3  p0 queue free %  99  96  99  cM capacity (veh/h)  1427  659  898  Direction, Lane #  EB 1 WB 1 NB 1  Volume Total  Volume Right  10  0  16  24  Volume Right  10  0  1427  703  Volume to Capacity  0.09  0.01  0.04  Queue Length (ft)  0  1  3  Control Delay (s)  A  Approach Delay (s)  Approach LOS  B  Intersection Summary  Average Delay  None										
Right turn flare (veh)  Median type  Median storage veh)  VC, conflicting volume  vC1, stage 1 conf vol  VC2, stage 2 conf vol  tC, single (s)  tF (s)  2.2  3.5  3.3  p0 queue free %  99  96  99  cM capacity (veh/h)  1427  659  898  Direction, Lane #  EB 1 WB 1 NB 1  Volume Total  Volume Total  Volume Right  10  0  16  24  Volume Right  10  0  7  cSH  1700  1427  703  Volume to Capacity  0.09  0.01  0.04  Queue Length (ft)  0  1  3  Control Delay (s)  A B  Approach Delay (s)  Intersection Summary  Average Delay  154  328  149  None  N										
Median type         None           Median storage veh)         VC, conflicting volume         154         328         149           VC1, stage 1 conf vol         VC2, stage 2 conf vol         VC2, stage 2 conf vol         VC2, stage (s)         VC2, stage (s)         VC2, stage (s)         VC2, stage (s)         VC3, stage (s)         VC4, stage (s										
Median storage veh) VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol CC, single (s) tC, 2 stage (s) tF (s)						None				
VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol tC, single (s) tC, 2 stage (s) tF (s)						None				
VC1, stage 1 conf vol  VC2, stage 2 conf vol  tC, single (s)				15/		320	1/0			
VC2, stage 2 conf vol tC, single (s)				104		320	149			
tC, single (s) tC, 2 stage (s) tF (s) 2.2 3.5 3.3 p0 queue free % 99 96 99 CM capacity (veh/h) 1427 659 898  Direction, Lane # EB 1 WB 1 NB 1  Volume Total 154 163 31 Volume Left 0 16 24 Volume Right 10 0 7 cSH 1700 1427 703 Volume to Capacity 0.09 0.01 0.04 Queue Length (ft) 0 1 3 Control Delay (s) Approach Delay (s) Approach LOS B Intersection Summary Average Delay 1.3										
tC, 2 stage (s)  tF (s)				11		6.4	6.2			
tF (s) 2.2 3.5 3.3 p0 queue free % 99 96 99 cM capacity (veh/h) 1427 659 898  Direction, Lane # EB 1 WB 1 NB 1 Volume Total 154 163 31 Volume Left 0 16 24 Volume Right 10 0 7 cSH 1700 1427 703 Volume to Capacity 0.09 0.01 0.04 Queue Length (ft) 0 1 3 Control Delay (s) 0.0 0.8 10.4 Lane LOS A B Approach Delay (s) 0.0 0.8 10.4 Approach LOS B  Intersection Summary Average Delay 1.3				4.1		0.4	0.2			
p0 queue free % 99 96 99 cM capacity (veh/h) 1427 659 898  Direction, Lane # EB 1 WB 1 NB 1  Volume Total 154 163 31  Volume Left 0 16 24  Volume Right 10 0 7 cSH 1700 1427 703  Volume to Capacity 0.09 0.01 0.04  Queue Length (ft) 0 1 3  Control Delay (s) 0.0 0.8 10.4  Lane LOS A B  Approach Delay (s) 0.0 0.8 10.4  Approach LOS B  Intersection Summary  Average Delay 1.3				2.2		3.5	3 3			
CM capacity (veh/h)       1427       659       898         Direction, Lane #       EB 1       WB 1       NB 1         Volume Total       154       163       31         Volume Left       0       16       24         Volume Right       10       0       7         cSH       1700       1427       703         Volume to Capacity       0.09       0.01       0.04         Queue Length (ft)       0       1       3         Control Delay (s)       0.0       0.8       10.4         Lane LOS       A       B         Approach Delay (s)       0.0       0.8       10.4         Approach LOS       B         Intersection Summary         Average Delay       1.3										
Direction, Lane # EB 1 WB 1 NB 1  Volume Total 154 163 31  Volume Left 0 16 24  Volume Right 10 0 7  cSH 1700 1427 703  Volume to Capacity 0.09 0.01 0.04  Queue Length (ft) 0 1 3  Control Delay (s) 0.0 0.8 10.4  Lane LOS A B  Approach Delay (s) 0.0 0.8 10.4  Approach LOS B  Intersection Summary  Average Delay 1.3										
Volume Total         154         163         31           Volume Left         0         16         24           Volume Right         10         0         7           cSH         1700         1427         703           Volume to Capacity         0.09         0.01         0.04           Queue Length (ft)         0         1         3           Control Delay (s)         0.0         0.8         10.4           Lane LOS         A         B           Approach Delay (s)         0.0         0.8         10.4           Approach LOS         B           Intersection Summary           Average Delay         1.3						000	030			
Volume Left         0         16         24           Volume Right         10         0         7           cSH         1700         1427         703           Volume to Capacity         0.09         0.01         0.04           Queue Length (ft)         0         1         3           Control Delay (s)         0.0         0.8         10.4           Lane LOS         A         B           Approach Delay (s)         0.0         0.8         10.4           Approach LOS         B           Intersection Summary           Average Delay         1.3										
Volume Right         10         0         7           cSH         1700         1427         703           Volume to Capacity         0.09         0.01         0.04           Queue Length (ft)         0         1         3           Control Delay (s)         0.0         0.8         10.4           Lane LOS         A         B           Approach Delay (s)         0.0         0.8         10.4           Approach LOS         B           Intersection Summary           Average Delay         1.3										
CSH 1700 1427 703  Volume to Capacity 0.09 0.01 0.04  Queue Length (ft) 0 1 3  Control Delay (s) 0.0 0.8 10.4  Lane LOS A B  Approach Delay (s) 0.0 0.8 10.4  Approach LOS B  Intersection Summary  Average Delay 1.3										
Volume to Capacity         0.09         0.01         0.04           Queue Length (ft)         0         1         3           Control Delay (s)         0.0         0.8         10.4           Lane LOS         A         B           Approach Delay (s)         0.0         0.8         10.4           Approach LOS         B           Intersection Summary           Average Delay         1.3										
Queue Length (ft)       0       1       3         Control Delay (s)       0.0       0.8       10.4         Lane LOS       A       B         Approach Delay (s)       0.0       0.8       10.4         Approach LOS       B         Intersection Summary         Average Delay       1.3										
Control Delay (s) 0.0 0.8 10.4  Lane LOS A B  Approach Delay (s) 0.0 0.8 10.4  Approach LOS B  Intersection Summary  Average Delay 1.3										
Lane LOS         A         B           Approach Delay (s)         0.0         0.8         10.4           Approach LOS         B           Intersection Summary           Average Delay         1.3										
Approach Delay (s) 0.0 0.8 10.4 Approach LOS B Intersection Summary Average Delay 1.3		0.0								
Approach LOS B Intersection Summary Average Delay 1.3										
Intersection Summary Average Delay 1.3		0.0	8.0							
Average Delay 1.3	Approach LOS			В						
	Intersection Summary									
Intersection Capacity Utilization 20.5% ICU Level of Service A	Average Delay									
	Intersection Capacity Uti	lization		20.5%	I	CU Leve	of Service		Α	

Intersection of Lamartine St & Grosvenor St AM peak Future Build Conditions									
	-	*	•	4	1	<b>/</b>			
Movement	EBT	EBR	WBL	WBT	NBL	NBR			
Lane Configurations	13			ર્ન	A				
Sign Control	Free			Free	Stop				
Grade	0%			0%	0%				
Volume (veh/h)	77	1	6	124	9	5			
Peak Hour Factor	0.92	0.25	0.42	0.86	0.75	0.50			
Hourly flow rate (veh/h)	84	4	14	144	12	10			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume			88		258	86			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)			4.1		6.4	6.2			
tC, 2 stage (s)									
tF (s)			2.2		3.5	3.3			
p0 queue free %			99		98	99			
cM capacity (veh/h)			1508		723	973			
Direction, Lane #	EB 1	WB 1	NB 1						
Volume Total	88	158	22						
Volume Left	0	14	12						
Volume Right	4	0	10						
cSH	1700	1508	819						
Volume to Capacity	0.05	0.01	0.03						
Queue Length (ft)	0	1	2						
Control Delay (s)	0.0	0.7	9.5						
Lane LOS		Α	Α						
Approach Delay (s)	0.0	0.7	9.5						
Approach LOS			А						
Intersection Summary									
Average Delay			1.2						
Intersection Capacity Uti	lization		20.0%	10	CU Leve	of Service		Α	

Intersection of Lama	artine S	St & G	rosven	or St F	PM pea	ak Future I	Build Condition	ons	
	-	*	1	4	1	-			
Movement	EBT	EBR	WBL	WBT	NBL	NBR			
Lane Configurations	1			4	Y				
Sign Control	Free			Free	Stop				
Grade	0%			0%	0%				
Volume (veh/h)	116	3	2	138	12	4			
Peak Hour Factor	0.78	0.25	0.50	0.85	0.63	0.50			
Hourly flow rate (veh/h)	149	12	4	162	19	8			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume			161		325	155			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)			4.1		6.4	6.2			
tC, 2 stage (s)									
tF (s)			2.2		3.5	3.3			
p0 queue free %			100		97	99			
cM capacity (veh/h)			1418		667	891			
Direction, Lane #	EB 1	WB 1	NB 1						
Volume Total	161	166	27						
Volume Left	0	4	19						
Volume Right	12	0	8						
cSH	1700	1418	721						
Volume to Capacity	0.09	0.00	0.04						
Queue Length (ft)	0	0	3						
Control Delay (s)	0.0	0.2	10.2						
Lane LOS		Α	В						
Approach Delay (s)	0.0	0.2	10.2						
Approach LOS			В						
Intersection Summary									
Average Delay			0.9			ng genge av			
Intersection Capacity Uti	lization		19.2%	I	CU Leve	l of Service		Α	

Intersection of Lafayette St & Meade St AM Peak Future Build Conditions									
	•	<b>→</b>	←	*	1	4			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations		र्स	1>		**				
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Volume (veh/h)	8	41	0	0	3	0			
Peak Hour Factor	0.85	0.80	0.92	0.92	0.33	0.33			
Hourly flow rate (veh/h)	9	51	0	0	9	0			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume	0				70	0			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)	4.1				6.4	6.2			
tC, 2 stage (s)									
tF (s)	2.2				3.5	3.3			
p0 queue free %	99				99	100			
cM capacity (veh/h)	1623				929	1085			
Direction, Lane #	EB 1	WB 1	SB 1						
Volume Total	61	0	9						
Volume Left	9	0	9						
Volume Right	0	0	0						
cSH	1623	1700	929						
Volume to Capacity	0.01	0.00	0.01						
Queue Length (ft)	0	0	1						
Control Delay (s)	1.2	0.0	8.9						
Lane LOS	Α		Α						
Approach Delay (s)	1.2	0.0	8.9						
Approach LOS			Α						
Intersection Summary									
Average Delay			2.2						
Intersection Capacity Uti	ilization		13.3%	IC	CU Leve	el of Service		Α	

Intersection of Lafay	Intersection of Lafayette St & Meade St PM Peak Future Build Conditions									
	۶	<b>→</b>	<b>←</b>	•	-	4				
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations		ર્ન	1		A					
Sign Control		Free	Free		Stop					
Grade		0%	0%		0%					
Volume (veh/h)	4	70	0	0	10	0				
Peak Hour Factor	0.75	0.76	0.92	0.92	0.50	0.33				
Hourly flow rate (veh/h)	5	92	0	0	20	0				
Pedestrians										
Lane Width (ft)										
Walking Speed (ft/s)										
Percent Blockage										
Right turn flare (veh)										
Median type					None					
Median storage veh)										
vC, conflicting volume	0				103	0				
vC1, stage 1 conf vol										
vC2, stage 2 conf vol										
tC, single (s)	4.1				6.4	6.2				
tC, 2 stage (s)										
tF (s)	2.2				3.5	3.3				
p0 queue free %	100				98	100				
cM capacity (veh/h)	1623				892	1085				
Direction, Lane #	EB 1	WB 1	SB 1							
Volume Total	97	0	20							
Volume Left	5	0	20							
Volume Right	0	0	0							
cSH	1623	1700	892							
Volume to Capacity	0.00	0.00	0.02							
Queue Length (ft)	0	0	2							
Control Delay (s)	0.4	0.0	9.1							
Lane LOS	Α		Α							
Approach Delay (s)	0.4	0.0	9.1							
Approach LOS			Α							
Intersection Summary										
Average Delay			1.9							
Intersection Capacity Uti	ilization		15.1%	IC	CU Leve	of Service		Α		

Intersection of Lafayette St & Grosvenor St AM Peak Future Build Conditions									
	۶	<b>→</b>	•	•	-	✓			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			Í
Lane Configurations		4	1		N.				
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Volume (veh/h)	11	43	0	0	7	0			
Peak Hour Factor	0.75	0.75	0.92	0.92	0.62	0.33			
Hourly flow rate (veh/h)	15	57	0	0	11	0			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume	0				87	0			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)	4.1				6.4	6.2			
tC, 2 stage (s)									
tF(s)	2.2				3.5	3.3			
p0 queue free %	99				99	100			
cM capacity (veh/h)	1623				906	1085			
Direction, Lane #	EB 1	WB1	SB 1						
Volume Total	72	0	11						
Volume Left	15	0	11						
Volume Right	0	0	0						
cSH	1623	1700	906						
Volume to Capacity	0.01	0.00	0.01						
Queue Length (ft)	1	0	1						
Control Delay (s)	1.5	0.0	9.0						
Lane LOS	Α		Α						
Approach Delay (s)	1.5	0.0	9.0						
Approach LOS			Α						
Intersection Summary									
Average Delay			2.5						
Intersection Capacity Uti	ilization		13.8%	IC	CU Leve	el of Service		Α	

Intersection of Lafayette St & Grosvenor St PM Peak Future Build Conditions									
	٠	<b>→</b>	<b>←</b>	•	-	1			
Movement	EBL	EBT	WBT	WBR	SBL	SBR		i i	
Lane Configurations		4	1		M				
Sign Control		Free	Free		Stop				
Grade		0%	0%		0%				
Volume (veh/h)	10	67	0	0	6	0			
Peak Hour Factor	0.50	0.80	0.92	0.92	0.50	0.33			
Hourly flow rate (veh/h)	20	84	0	0	12	0			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume	0				124	0			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)	4.1				6.4	6.2			
tC, 2 stage (s)									
tF (s)	2.2				3.5	3.3			
p0 queue free %	99				99	100			
cM capacity (veh/h)	1623				861	1085			
Direction, Lane #	EB 1	WB 1	SB 1					1	
Volume Total	104	0	12						
Volume Left	20	0	12						
Volume Right	0	0	0						
cSH	1623	1700	861						
Volume to Capacity	0.01	0.00	0.01						
Queue Length (ft)	1	0	1						
Control Delay (s)	1.5	0.0	9.2						
Lane LOS	Α		Α						
Approach Delay (s)	1.5	0.0	9.2						
Approach LOS			Α						
Intersection Summary									
Average Delay			2.3						
Intersection Capacity Uti	lization		15.5%	IC	CU Leve	el of Service	А		

Intersection of Lama	Intersection of Lamartine St, Hermon St, Lodi St & Green St AM Peak Future Build Conditions											
	٠	<b>→</b>	*	•	-	*	1	<b>†</b>	-	1	<b>↓</b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	30	88	43	15	64	9	7	61	66	7	37	9
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.85	0.85	0.85	0.82	0.82	0.82
Hourly flow rate (veh/h)	34	101	49	16	69	10	8	72	78	9	45	11
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	185	95	158	65								
Volume Left (vph)	34	16	8	9								
Volume Right (vph)	49	10	78	11								
Hadj (s)	-0.1	0.0	-0.3	0.0								
Departure Headway (s)	4.4	4.7	4.3	4.7								
Degree Utilization, x	0.23	0.12	0.19	0.08								
Capacity (veh/h)	771	567	781	729								
Control Delay (s)	8.7	8.3	8.4	8.1								
Approach Delay (s)	8.7	8.3	8.4	8.1								
Approach LOS	Α	Α	А	Α								
Intersection Summary												
Delay			8.4									
HCM Level of Service			Α									
Intersection Capacity Uti	lization		33.5%	10	CU Leve	el of Ser	vice		Α			

## Intersection of Lamartine St, Ggreen Island Blvd, Lodi and Hermon Sts PM Peak Future Build Conditions

	۶	<b>→</b>	*	1	•	*	1	1	-	-	<b>↓</b>	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	701
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	8	57	35	25	37	4	16	54	80	18	114	19
Peak Hour Factor	0.89	0.89	0.89	0.79	0.79	0.79	0.81	0.81	0.81	0.76	0.76	0.76
Hourly flow rate (veh/h)	9	64	39	32	47	5	20	67	99	24	150	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	112	84	185	199								
Volume Left (vph)	9	32	20	24								
Volume Right (vph)	39	5	99	25								
Hadj (s)	-0.2	0.1	-0.3	0.0								
Departure Headway (s)	4.7	4.7	4.3	4.5								
Degree Utilization, x	0.15	0.11	0.22	0.25								
Capacity (veh/h)	711	566	794	763								
Control Delay (s)	8.5	8.3	8.5	9.0								
Approach Delay (s)	8.5	8.3	8.5	9.0								
Approach LOS	Α	Α	А	Α								
Intersection Summary												
Delay			8.7									
HCM Level of Service			Α									
Intersection Capacity Uti	lization		29.0%	10	CU Leve	el of Ser	vice		Α			